

# Moore Lake Stormwater Retrofit Analysis

Prepared by:



*for the* RICE CREEK WATERSHED DISTRICT

May 2013

**Cover photos:** Aerial photographs of Moore Lake from 1938, 1953, and 2011. The 1938 photograph shows the landscape prior to the construction of Highway 65 through the middle of Moore Lake, while the 1953 photograph shows partial completion of the highway through the northern part of the lake.

## Contents

Stormwater Catchment Map
Executive Summary
About this Document
Catchment Profiles9
Section 1: Directly Connected Catchments
Catchment ML-1
Catchment ML-519
Catchment ML-627
Section 2: North Network
Catchment ML-2
Catchment ML-341
Catchment ML-447
Section 3: Disconnected Catchments
Catchment ML-751
Catchment ML-853
Catchment ML-956
Catchment ML-1058
Retrofit Ranking60
References
Appendix A – Methods
Appendix B – How to Read Catchment Profiles

Appendix C – Rain Garden Concept Designs

Appendix D – Retrofit Concept Designs



### Map of stormwater catchments referred to in this report.

## **Executive Summary**

This study provides recommendations for cost effectively improving treatment of stormwater from areas draining to Moore Lake (herein described as East and West Moore Lake when differentiating basins), which is located within the Rice Creek Watershed District in the City of Fridley. The lake is classified as a high priority water body by the Rice Creek Watershed District and is listed as a Tier II water body by the district. Tier II water bodies provide passive regional public recreation opportunities, and Moore Lake is used recreationally for swimming and fishing. Panfish are the primary sport fish sought by anglers, which also results in frequent ice fishing during the winter months.

East Moore Lake was added to the EPA's 303(d) list of impaired waters in 2002 for aquatic recreation due to excess nutrients. Years of water quality data have indicated high concentrations of phosphorus and chlorophyll a. Efforts to improve water quality within Moore Lake have resulted in a long history of lake and subwatershed activities. These have included numerous efforts within the subwatershed to treat stormwater runoff before it enters the lake as well as the installation of a plastic liner on the bottom of East Moore Lake to reduce sediment resuspension. The stormwater retrofits in this report will aid with alleviating existing water quality problems in Moore Lake.

East and West Moore Lakes are bisected by Highway 65 and are connected by a culvert under the highway. During periods of high water the lake outlets to the north via an outlet on the north side of West Moore Lake. The subwatershed consists of 936 acres (not including the lake area) and is dominated by medium density residential and commercial land uses. Of the 936 acre subwatershed, 659 acres are connected to Moore Lake via overland flow or stormwater infrastructure.

This stormwater analysis focuses on "stormwater retrofitting" and ranking projects on cost effectiveness. Stormwater retrofitting refers to adding stormwater treatment to an already built-up area, where little open land exists. This process is investigative and creative. Stormwater retrofitting success is sometimes improperly judged by the number of projects installed or by comparing costs alone. Those approaches neglect to consider how much pollution is removed per dollar spent. In this stormwater analysis we estimated both costs and pollutant reductions and used them to calculate cost effectiveness of each possible project.

Areas that drain to Moore Lake were delineated using available GIS subwatershed information and maps of stormwater conveyance features. Those areas were then divided into 10 smaller stormwater drainage areas, or catchments. For each catchment, modeling of stormwater volume and pollutants was completed using the software WinSLAMM. Base and existing conditions were modeled, including existing stormwater treatment practices. The total subwatershed analyzed for this project consisted of 936 acres. The 659 acres connected to Moore Lake contribute an estimated 395 acre feet of runoff, 392 pounds of phosphorus, and 83,112 pounds of total suspended solids each year.

Potential stormwater retrofits identified during this analysis were then modeled to estimate reductions in volume, total phosphorus, and total suspended solids. Finally, cost estimates were developed for

each retrofit project, including 30 years of operations and maintenance. Projects were ranked by cost effectiveness with respect to their reduction of total phosphorus.

A variety of stormwater retrofit approaches were identified. They included:

- Maintenance of, or alterations to, existing stormwater treatment practices,
- Residential curb-cut rain gardens,
- New stormwater pond opportunities,
- Permeable pavement,
- Hydrodynamic separators, and
- Stormwater redirection.

If all of these practices were installed, significant pollution reduction could be accomplished. However, funding limitations and landowner interest makes this goal unlikely. Instead, it is recommended that projects be installed in order of cost effectiveness (pounds of pollution reduced per dollar spent). Other factors, including a project's educational value/visibility, construction timing, total cost, or non-target pollutant reduction also affect project installation decisions and will need to be weighed by resource managers when selecting projects to pursue.

This report provides conceptual sketches or photos of recommended stormwater retrofitting projects. The intent is to provide an understanding of the approach. If a project is selected, site-specific designs must be prepared. In addition, many of the proposed retrofits (e.g. wet ponds) will require engineered plan sets if selected. This typically occurs after committed partnerships are formed to install the project. Committed partnerships must include willing landowners when installed on private property.

The tables on the next pages summarize potential projects. Potential projects are organized from most cost effective to least, based on cost per pound of total phosphorus removed. Installation of projects in series will result in lower total treatment than the simple sum of treatment across the individual projects due to treatment train effects. Reported treatment levels are dependent upon optimal site selection and sizing. More detail about each project can be found in the catchment profile pages of this report. Projects that were deemed unfeasible due to prohibitive size, number, or were too expensive to justify installation are not included in this report.

Catchments 1 through 6: Summary of preferred stormwater retrofit opportunities ranked by cost-effectiveness with respect to total phosphorus (TP) reduction. Volume and total suspended solids (TSS) reductions are also shown. For more information on each project refer to the catchment profile

pages	in this repo	brt.						-		
Project Rank	Catchment ID	Retrofit Type (refer to catchment profile pages for additional detail)	Projects Identified	TP Reduction (lb/yr)	TSS Reduction (Ib/yr)	Volume Reduction (ac-ft/yr)	Total Project Cost	Estimated Annual Operations & Maintenance (2013 Dollars)	Estimated cost/ 1,000lb-TSS (30-year)	Estimated cost/ lb-TP (30-year)
1	ML-5	New Wet Pond (East Moore Lake Park)	1	15.5	5972	0.0	\$51,177	\$700	\$403	\$155
2	ML-5	Residential Rain Gardens	5 - 20	5.0 - 15.1	1,425 - 4,221	3.5 - 10.3	\$38,761 - \$144,421	\$375 - \$1,500	\$1,170 - \$1,496	\$333 - \$418
ε	ML-1	Residential Rain Gardens	1 - 5	1.1 - 4.1	307 - 1,158	0.8 - 2.8	\$10,585 - \$38,761	\$75 - \$375	\$1,346 - \$1,440	\$388 - \$407
4	ML-3	Residential Rain Gardens	5 - 15	3.3 - 8.1	721 - 1,822	3.4 - 8.2	\$34,381 - \$96,061	\$375 - \$1,125	\$2,110 - \$2,375	\$461 - \$534
ß	9-1W	Residential Rain Gardens (downstream of pond)	3	1.9	508	1.2	\$22,045	\$225	\$1,889	\$505
9	ML-2	Residential Rain Gardens	3 - 10	1.9 - 5.0	422 - 1,127	2.0 - 4.9	\$22,045 - \$65,221	\$225 - \$750	\$2,274 - \$2,595	\$505 - \$585
7	9-1W	Rain Garden (Elementary School)	1	0.7 - 1.7	118 - 282	1.0 - 2.0	\$10,446 - \$27,396	\$75	\$3,073 - \$3,586	\$510 - \$605
8	ML-4	Residential Rain Gardens	10 - 30	4.6 - 11.5	862 - 2,200	6.9 - 16.4	\$65,221 - \$188,581	\$750 - \$2,250	¢3,392 - \$3,880	\$636 - \$742
6	9-1W	Iron Enhanced Sand Filter Pond Modification	1	4.3	0	0.0	\$48,252	\$1,550	N/A	\$735
10	ML-3	Pond Modification (Creekridge Park)	1	2.0	677	0.0	\$32,109	\$450	\$2,246	\$760
11	9-1W	Residential Rain Gardens (upstream of pond)	5 - 10	1.8 - 3.3	263 - 494	3.3 - 5.8	\$34,381 - \$65,221	<b>\$375 - \$750</b>	\$5,783 - \$5,919	\$845 - \$886
12	ML-1	Hydrodynamic Device (59th)	1	0.5 - 0.9	224 - 392	0.0	\$18,252 - \$46,752	\$420	\$3,900 - \$5,047	\$1,755 - \$2,198
13	ML-1	Grass Swale (Church)	1	0.4	181	0.2	\$14,896	Ş584	\$5,970	\$2,701
14	ML-3	New Wet Pond (Old Central Ave.)	1	2.0	657	0.0	\$164,548	\$450	\$9,033	\$2,967
15	ML-1	Hydrodynamic Device (58th)	1	0.3 - 0.5	113 - 199	0.0	\$18,252 - \$46,752	\$420	\$7,824 - \$9,942	\$3,071 - \$3,957
Pollution r	eduction benefits a	and costs can not be summed with other projects in the sam	he catchment be	cause they are a	Iternative option	is for treating th€	same source area.			
		Directly Connected Catchment								

Executive Summary

Moore Lake Stormwater Retrofit Analysis

North Network

Catch (TP) pages	iments 1 th reduction. in this repo	<pre>trough 6: Summary of preferred s Volume and total suspended solids ort.</pre>	stormwa (TSS) rec	<b>ter retrof</b> luctions ai	<b>it opport</b> u re also shc	unities rai wn. For i	<b>nked by cost-</b> e more informati	e <b>ffectiveness with</b> ion on each project	r <b>respect to total I</b> refer to the catchr	<b>hosphorus</b> nent profile
Project Rank	Catchment ID	Retrofit Type (refer to catchment profile pages for additional detail)	Projects Identified	TP Reduction (lb/yr)	TSS Reduction (lb/yr)	Volume Reduction (ac-ft/yr)	Total Project Cost	Estimated Annual Operations & Maintenance (2013 Dollars)	Estimated cost/ 1,000lb-TSS (30-year)	Estimated cost/ lb-TP (30-year)
16	9-TW	Hydrodynamic Device (Polk)	1	50 - 6.0	101 - 176	0.0	\$18,252 - \$46,752	\$420	\$8,774 - \$11,241	\$3,071 - \$3,957
17	ML-2	Hydrodynamic Device	1	0.3 - 0.4	71 - 130	0.0	\$18,252 - \$46,752	\$420	\$11,270 - \$15,218	\$3,071 - \$4,946
18	ML-2	New Wet Pond (East Moore Lake Park)	1	2.4	701	0.0	\$223,244	\$450	\$11,257	\$3,288
19	ML-1	Permeable Asphalt (Fridley High School)	1	1.9	632	1.7	\$208,662	\$476	\$11,758	\$3,911
20	ML-5	Permeable Asphalt (Totino Grace)	1	1.7	1,001	2.8	\$219,552	\$501	\$7,812	\$4,600
21	ML-1	Permeable Asphalt (Church)	1	1.0	615	1.7	\$136,752	\$311	\$7,917	\$4,869
22	9-TW	Permeable Asphalt (High Rise Residential 0.15 acre)	1	0.5	300	0.8	\$68,968	\$150	\$8,164	\$4,898
23	9-TW	Permeable Asphalt (High Rise Residential 0.1 acre)	1	0.3	200	0.6	\$47,188	\$100	\$8,366	\$5,577
24	ML-5	Grass Swale (East Moore Lake Park)	1	0.1	68	0.1	\$7,464	\$584	\$12,247	\$8,328
25	WL-3	New Wet Pond (Rice Creek Rd.)	1	9.0	251	0.0	\$240,448	\$450	\$33,725	\$14,108
Pollution	reduction benefits	and costs can not be summed with other projects in the san	ne catchment b	ecause they are a	alternative optior	is for treating the	e same source area.			



North Network

Catchment 8: Summary of preferred stormwater retrofit opportunities ranked by cost-effectiveness with respect to total phosphorus (TP) reduction. Retrofits within catchment 8 will benefit Innsbruck Nature Center. Volume and total suspended solids (TSS) reductions are also shown. For more information on each project refer to the catchment profile pages in this report.

Estimated cost/ lb-TP (30-year)	\$960 - \$1,014	
Estimated cost/ 1,000lb-TSS (30-year)	\$6,315 - \$6,873	
Estimated Annual Operations & Maintenance (2013 Doilars)	\$75 - \$375	
Total Project Cost	\$9,709 - \$34,38 <b>1</b>	
Volume Reduction (ac-ft/yr)	0.7 - 2.1	
TSS Reduction (lb/yr)	58 - 226	
TP Reduction (lb/yr)	0.4 - 1.5	
Projects Identified	1 - 5	
Retrofit Type (refer to catchment profile pages for additional detail)	Residential Rain Gardens	
Catchment ID	WL-8	
Project Rank	1	

Disconnected Catchment

6

**Executive Summary** 

Moore Lake Stormwater Retrofit Analysis

# **About this Document**

This Stormwater Retrofit Analysis is a watershed management tool to help prioritize stormwater retrofit projects by performance and cost effectiveness. This process helps maximize the value of each dollar spent.

### **Document Organization**

This document is organized into three major sections, plus references and appendices. Each section is briefly described below.

### Methods

The methods section outlines general procedures used when analyzing the subwatershed. It overviews the processes of retrofit scoping, desktop analysis, retrofit reconnaissance investigation, cost/treatment analysis, and project ranking. See Appendix A for a detailed description of the methods.

### **Catchment Profiles**

The Moore Lake subwatershed was divided into stormwater catchments for the purpose of this analysis. See Appendix B for a guide to reading the catchment profiles. Each catchment was given a unique ID number. For each catchment, the following information is detailed:

### **Catchment Description**

Within each catchment profile is a table that summarizes basic catchment information including acres, land cover, parcels, and estimated annual pollutant and volume loads. A brief description of the land cover, stormwater infrastructure, and any other important general information is also described. Existing stormwater practices are noted, and their estimated effectiveness presented.

### **Retrofit Recommendations**

The recommendation section describes the conceptual retrofit(s) that were scrutinized. It includes tables outlining the estimated pollutant removals by each, as well as costs. A map provides promising locations for each retrofit approach.

### **Retrofit Ranking**

This section ranks stormwater retrofit projects across all catchments to create a prioritized project list. The list is sorted by cost per pound of total phosphorus removed for each project over 30 years. The final cost per pound treatment value includes installation and maintenance costs.

There are many possible ways to prioritize projects, and the list provided in this report is merely a starting point. Other considerations for prioritizing installation may include:

- Non-target pollutant reductions
- Timing projects to occur with other road or utility work
- Project visibility
- Availability of funding
- Total project costs
- Educational value

#### References

This section identifies various sources of information synthesized to produce the protocol utilized in this analysis.

### **Appendices**

This section provides supplemental information and/or data used during the analysis.

Map of stormwater networks and catchment areas referred to in this report. Catchment profiles on the following pages provide additional detail.



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## **Section 1: Directly Connected Catchments**

Existing Directly Connect	ed Summary
Acres	284
Dominant Land Cover	Residential
Parcels	610
TP (lbs/yr)	180.5
TSS (lbs/yr)	44,203
Volume (acre-feet/yr)	171.9

### DIRECTLY CONNECTED CATCHMENTS

Catchment ID	Page
ML-1	12
ML-5	19
ML-6	27



#### **EXISTING TREATMENT**

The image below represents a simplified flow network for the catchments that are directly connected to Moore Lake. Stormwater infrastructure throughout the directly connected catchments consists primarily of pipes. Existing stormwater treatment consists of several treatment ponds, curb-cut rain gardens, and street sweeping. In addition, a biologically activated soil filtration unit (BASFU) is located in catchment ML-6, though it is no longer functional.



## **Catchment ML-1**

Existing Catchment S	ummary
Acres	92
Dominant Land Cover	Residential
Parcels	129
TP (lbs/yr)	50.9
TSS (lbs/yr)	11,981
Volume (acre-feet/yr)	57.1

#### **CATCHMENT DESCRIPTION**

Catchment ML-1 consists of nine different land use types (freeway, institutional, medium density residential, multi-family residential, office park, open space, park, school, and open water). The area directly connected to West Moore Lake is encompassed within a relatively narrow fringe (0.1 – 0.2 miles wide) around the lake. It contains



portions of the Fridley Middle School and Medtronic campuses and Moore Lake Park West. The entire catchment drains to West Moore Lake via stormwater infrastructure, and stormwater enters the lake through multiple inlets.

#### **EXISTING STORMWATER TREATMENT**

In addition to street sweeping by the City of Fridley, scattered stormwater treatment exists throughout catchment ML-1. The most substantial treatment exists on the Medtronic campus, which consists of three stormwater treatment ponds. Four curb-cut rain gardens also exist on the Fridley Middle School campus, which capture runoff from the campus as well as connected streets. The remaining untreated stormwater enters West Moore Lake via overland flow and more directed inlets such as stormwater pipes. The table below shows the base and existing conditions as well as how existing treatment practices within catchment ML-1 affect the stormwater volume and pollutant loads entering West Moore Lake.

#### **Existing Conditions**

	Existing Conditions	Base Loading	Treatment	Net Treatment %	Existing Loading
	TP (lb/yr)	68.7	17.8	26%	50.9
nt	TSS (lb/yr)	18,856	6,875.0	36%	11,981
me	Volume (acre-feet/yr)	66.5	9.4	14%	57.1
eat	Number of BMP's		8	3	
L I	BMP Size/Description	Medtron	ic ponds, Frid gardens, stre	ley Middle Sc eet sweeping	hool rain

### **RETROFIT RECOMMENDATIONS**



#### Project ID: ML-1 Residential Rain Gardens

#### Drainage Area – 21.7 acres

Location - Southwest corner of catchment ML-1 within residential land use

#### Property Ownership - Private

**Description** – Very little space is available for retrofits in this catchment. However, there are some opportunities to install curb-cut rain gardens to treat the residential land use (see Appendix C for design options). Six ideal rain garden locations were identified (see map), though more may exist. Generally, ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, scenarios with 1, 3, and 5 rain gardens were analyzed to treat the residential land use. Catchment-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

#### Conceptual images -



Before/24 -48 hours after rain



During rain

#### **Curb-Cut Rain Gardens Treating Residential Land Use**

				Proje	ct ID		
		1 Curb-	Cut Rain	3 Curb-0	Cut Rain	5 Curb-	Cut Rain
	Cost/Removal Analysis	Gar	den	Gard	dens	Gare	dens
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %
	TP (lb/yr)	1.1	28%	2.7	30%	4.1	32%
	TSS (lb/yr)	307	38%	778	41%	1,158	43%
	Volume (acre-feet/yr)	0.8	15%	1.9	17%	2.8	18%
ent	Number of BMP's		1		3	I	5
Treatm	BMP Size/Description	250	square feet	750	square feet	1,250	square feet
	ВМР Туре	Complex B	ioretention	Complex B	oretention	Complex B	ioretention
	Materials/Labor/Design	\$5,	876	\$17	,628	\$29	,380
*	Promotion & Admin Costs	\$4,	709	\$7,045		\$9,381	
Sost	Probable Project Cost	\$10	,585	\$24,673		\$38	,761
	Annual O&M	\$	75	\$2	25	\$3	75
	30-yr Cost/lb-TP	\$3	89	\$3	88	\$4	07
	30-yr Cost/1,000lb-TSS	\$1,	394	\$1,	346	\$1,	440

#### Project ID: ML-1 Grass Swale at St. Philip's Lutheran Church

#### Drainage Area – 2.0 acres

Location – Southwest corner of W Moore Lake Dr. and Highway 65 intersection

#### Property Ownership – Private

**Description** – St. Philip's Lutheran Church is located on the north side of West Moore Lake. The western side of the campus drains south through a curb-cut before entering West Moore Lake. Space is available to construct a vegetated swale that will treat stormwater before it enters the lake. Volume reduction and removal of TP and TSS are shown in the table below.

### Project ID: ML-1 Permeable Asphalt at St. Philip's Lutheran Church

#### Drainage Area - 1.2 acres

Location - Southwest corner of W Moore Lake Dr. and Highway 65 intersection

#### Property Ownership – Private

**Description** – St. Philip's Lutheran Church is located on the north side of West Moore Lake. The western side of the campus drains south through a curb-cut before entering West Moore Lake. Stormwater runoff produced by the parking lot could be treated using permeable asphalt (see Appendix D for design options) prior to it entering the curb-cut. A detailed investigation of soils at the site would be necessary to determine acceptable infiltration capacity because of the close proximity to the lake and potentially high water table. Volume reduction and removal of TP and TSS are shown in the table below.

				Proje	ect ID	
	Cost/Removal Analysis	130' Gra	ss Swale	0.3 a Permeabl	cres e Asphalt	
		New trtmt	Net %	New trtmt	Net %	
	TP (lb/yr)	0.4	26%	1.0	27%	
	TSS (lb/yr)	181	37%	615	40%	
	Volume (acre-feet/yr)	0.2	14%	1.7	17%	
ent	Number of BMP's		1	1	L	
reatm	BMP Size/Description	2,080	square feet	13,500	square feet	
F	ВМР Туре	Dry S	Swale	Permeabl	e Asphalt	
	Materials/Labor/Design	\$13	,728	\$135	,000	
	Promotion & Admin Costs	\$1,	168	\$1,	752	
Cost	Probable Project Cost	\$14	,896	\$136	,752	
	Annual O&M	\$5	84	\$3	11	
	30-yr Cost/lb-TP	\$2,	701	\$4,	869	
	30-yr Cost/1,000lb-TSS	\$5,	970	\$7,	917	

#### **Church Parking Lot Grass Swale and Permeable Asphalt**

#### Project ID: ML-1 Permeable Asphalt at Fridley High School

Drainage Area – 1.9 acres

*Location* – Fridley High School parking lot west of W Moore Lake Dr.

Property Ownership – Private

**Description** – Fridley High School is located on the west side of W Moore Lake. Stormwater runoff from the parking lot that borders W Moore Lake Dr. could be treated using permeable asphalt (see Appendix D for design options) prior to it entering the curb-cut. A detailed investigation of soils at the site would be necessary to determine acceptable infiltration capacity because of the close proximity to the lake and potentially high water table. Volume reduction and removal of TP and TSS are shown in the table below.

				Proje	ct ID	
	Cost/Removal Analysis	0.475 Permeab	acres le Asphalt			
		New trtmt	Net %			
	TP (lb/yr)	1.9	29%			
	TSS (lb/yr)	632	40%			
	Volume (acre-feet/yr)	1.7	17%			
ent	Number of BMP's		1			
Treatm	BMP Size/Description	20,691 square feet				
	ВМР Туре	Permeab	le Asphalt			
	Materials/Labor/Design	\$206	5,910			
<b>A</b> .	Promotion & Admin Costs	\$1,752				
Sos	Probable Project Cost	\$208	3,662			
	Annual O&M	\$4	76			
	30-yr Cost/lb-TP	\$3,	911			
	30-yr Cost/1,000lb-TSS	\$11	,758			

#### Fridley High School Permeable Asphalt

### Project ID: ML-1 Hydrodynamic Separator (near 59th Ave. NE)

Drainage Area – 9.2 acres

*Location* – Near intersection of W Moore Lake Dr. and 59<sup>th</sup> Ave NE.

Property Ownership – City of Fridley

**Description** – The confluence of multiple stormwater lines within catchment ML-1 near the intersection of W Moore Lake Dr. and 59<sup>th</sup> Ave. NE is a potential site for a hydrodynamic separator (see Appendix D for additional information). The structural best management practice uses hydrodynamic separation to remove particulate pollutants from stormwater and could be installed in place of an existing catch basin. Scenarios with 4, 6, and 8 foot diameter devices were analyzed to treat the residential and institutional land use. Removal of TP and TSS could be increased to the levels shown in the following table.

				Proje	ect ID		
	Cost/Removal Analysis	Hydrod Sepa	lynamic rator	Hydrod Sepa	ynamic rator	Hydrod Sepa	lynamic rator
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %
	TP (lb/yr)	0.9	27%	0.7	27%	0.5	27%
	TSS (lb/yr)	392	39%	315	38%	224	38%
	Volume (acre-feet/yr)	0.0	14%	0.0	14%	0.0	14%
ent	Number of BMP's		1	-	L		1
reatm	BMP Size/Description	8 foot diameter		6	foot diameter	4	foot diameter
1	ВМР Туре	Hydrod Sepa	lynamic Irator	Hydrod Sepa	ynamic rator	Hydrod Sepa	lynamic rator
	Materials/Labor/Design	\$45	,000	\$22	,500	\$16	,500
	Promotion & Admin Costs	\$1,	\$1,752 \$1,752		752	\$1,	752
Sost	Probable Project Cost	\$46	,752	\$24	,252	\$18	,252
	Annual O&M	\$4	20	\$4	20	\$4	20
	30-yr Cost/lb-TP	\$2,	198	\$1,	755	\$2,	057
	30-yr Cost/1,000lb-TSS	\$5,	047	\$3,	900	\$4,	591

Hydrodynamic Separator (near 59<sup>th</sup> Ave. NE)

### Project ID: ML-1 Hydrodynamic Separator (near 58<sup>th</sup> Ave. NE)

Drainage Area – 6.4 acres

*Location* – Near intersection of W Moore Lake Dr. and 58<sup>th</sup> Ave NE.

Property Ownership – City of Fridley

**Description** – The confluence of multiple stormwater lines within catchment ML-1 near the intersection of W Moore Lake Dr. and 59<sup>th</sup> Ave. NE is a potential site for a hydrodynamic separator (see Appendix D for additional information). The structural best management practice uses hydrodynamic separation to remove particulate pollutants from stormwater and could be installed in place of an existing catch basin. Scenarios with 4, 6, and 8 foot diameter devices were analyzed to treat the residential land use. Volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

		Project ID						
	Cost/Removal Analysis	Hydrod Sepa	lynamic rator	Hydrod Sepa	ynamic rator	Hydrodynamic Separator		
	Cost/Removal Analysis TP (Ib/yr) TSS (Ib/yr) Volume (acre-feet/yr) Number of BMP's BMP Size/Description BMP Type Materials/Labor/Design Promotion & Admin Costs Probable Project Cost Annual O&M 30-yr Cost/Ib-TP 30-yr Cost/1 000Ib-TSS	New trtmt	Net %	New trtmt	Net %	New trtmt	Net %	
	TP (lb/yr)	0.5	27%	0.4	26%	0.3	26%	
	TSS (lb/yr)	199	38%	157	37%	113	37%	
	Volume (acre-feet/yr)	0.0	14%	0.0	14%	0.0	14%	
ent	Number of BMP's	1		1				
reatme	BMP Size/Description	8	foot diameter	6	foot diameter	4	foot diameter	
L	ВМР Туре	Hydrodynamic Separator		Hydrod Sepa	Hydrodynamic Separator		Hydrodynamic Separator	
	Materials/Labor/Design	\$45	,000	\$22	,500	\$16	,500	
4.1	Promotion & Admin Costs	\$1,	752	\$1,	752	\$1,	752	
Cost	Probable Project Cost	\$46	,752	\$24	,252	\$18	,252	
	Annual O&M	\$4	20	\$4	20	\$4	20	
	30-yr Cost/lb-TP	\$3,	957	\$3,	071	\$3,	428	
	30-yr Cost/1,000lb-TSS	\$9,	942	\$7,	824	\$9,	\$9,101	

#### Hydrodynamic Separator (near 58<sup>th</sup> Ave. NE)

## **Catchment ML-5**

Existing Catchment S	ummary
Acres	120
Dominant Land Cover	Residential, School
Parcels	245
TP (lbs/yr)	92.2
TSS (lbs/yr)	24,157
Volume (acre-feet/yr)	66.1

#### **CATCHMENT DESCRIPTION**

Catchment ML-5 consists of medium density residential, park land, open water, and school land uses. Most of the eastern half of the catchment is comprised of Totino Grace High School and its athletic fields and other associated structures, while the western half is primarily a residential neighborhood. Moore Lake Park and Beach occupies much of the shoreline along the lake.



#### **EXISTING STORMWATER TREATMENT**

Existing stormwater treatment within catchment ML-5 consists of rain gardens, stormwater treatment ponds on and nearby the Totino Grace High School campus, and street sweeping by the City of Fridley. Stormwater is generally conveyed from east to west into East Moore Lake via stormwater infrastructure. With the exception of one residential curb-cut rain garden, stormwater from the residential neighborhoods west of Totino Grace High School receives no treatment prior to entering East Moore Lake. The table below shows how existing treatment practices within catchment ML-5 affect the stormwater volume and pollutant loads entering East Moore Lake.

#### **Existing Conditions**

	Existing Conditions	Base Loading	Treatment	Net Treatment %	Existing Loading		
	TP (lb/yr)	105.5	13.3	13%	92.2		
nt	TSS (lb/yr)	28,475	4,318.0	15%	24,157		
me	Volume (acre-feet/yr)	69.8	3.7	5%	66.1		
eatr	Number of BMP's	6					
1	BMP Size/Description	Rain gardens, Totino Grace High School ponds, street sweeping					

### **RETROFIT RECOMMENDATIONS**



#### Project ID: ML-5 Residential Rain Gardens

#### Drainage Area – 63.4 acres

Location – Throughout catchment ML-5 in residential land use

#### Property Ownership - Private

**Description** – The residential nature of this catchment makes it best suited to curb-cut rain gardens (see Appendix C for design options). Twenty nine ideal rain garden locations were identified (see map), though more exist. Generally, ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, scenarios with 5, 10, and 20 rain gardens were analyzed to treat the residential land use. Catchment-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

#### Conceptual images -



Before/24 -48 hours after rain



During rain

#### Curb-Cut Rain Gardens Treating Residential Land Use

				Proje	ect ID		
	Cost/Removal Analysis	5 Curb-( Gar	Cut Rain den	10 Curb-Cut Rain Gardens		20 Curb-Cut Rain Gardens	
Cost Treatment		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %
	TP (lb/yr)	5.0	17%	9.0	21%	15.1	27%
	TSS (lb/yr)	1,425	20%	2,542	24%	4,221	30%
	Volume (acre-feet/yr)	3.5	10%	6.2	14%	10.3	20%
ent	Number of BMP's	5		10		20	
reatme	BMP Size/Description	1,250	square feet	2,500	square feet	5,000	square feet
L	ВМР Туре	Complex B	ioretention	Complex Bioretention		Complex Bioretention	
	Materials/Labor/Design	\$29	,380	\$58,760		\$117	7,520
	Promotion & Admin Costs	\$9,	381	\$15,221		\$26,901	
Sos	Probable Project Cost	\$38	,761	\$73,	,981	\$144	,421
	Annual O&M	\$3	75	\$7	50	\$1,	500
	30-yr Cost/lb-TP	\$3	33	\$3	57	\$4	18
	30-yr Cost/1,000lb-TSS	\$1,	170	\$1,265		\$1,496	

#### Project ID: ML-5 Permeable Asphalt at Totino Grace High School

Drainage Area – 2.0 acres

Location – Main parking lot on west side of Totino Grace High School

Property Ownership – Totino Grace High School

**Description** – Totino Grace High School is located on the eastern side of catchment ML-5. Stormwater runoff from the main parking lot could be treated using permeable asphalt (see Appendix D for design options) prior to it entering the stormwater infrastructure. Permeable asphalt is well suited to this area because of the large amount of impervious surface. Volume reduction and removal of TP and TSS are shown in the table below.

#### **Totino Grace High School Permeable Asphalt**

				Project ID				
		0.50	acres					
	Cost/Removal Analysis	Permeable Asphalt						
Cost Treatment		New trtmt	Net %					
	TP (lb/yr)	1.7	27%					
	TSS (lb/yr)	1,001	39%					
	Volume (acre-feet/yr)	2.8	18%					
ent	Number of BMP's	1						
reatme	BMP Size/Description	21,780	square feet					
T	ВМР Туре	Permeable Asphalt						
	Materials/Labor/Design	\$217	7,800					
4.	Promotion & Admin Costs	\$1,	752					
Cost	Probable Project Cost	\$219	),552					
	Annual O&M	\$5	01					
	30-yr Cost/lb-TP	\$4,	600					
	30-yr Cost/1,000lb-TSS	\$7,	812					

#### Project ID: ML-5 Hydrodynamic Separator (Hackman Ave. NE)

Drainage Area – 6.4 acres

Location – Near intersection of Hackman Ave. NE and Hackman Circle NE

**Property Ownership** – City of Fridley

**Description** – The confluence of multiple stormwater lines within catchment ML-5 near the intersection of Hackman Ave. NE and Hackman Circle NE is a potential site for a hydrodynamic separator (see Appendix D for additional information). The structural best management practice uses hydrodynamic separation to remove particulate pollutants from stormwater and could be installed in place of an existing catch basin. Scenarios with 4, 6, and 8 foot diameter devices were analyzed to treat the residential land use. Volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

				Proje	ct ID		
		Hydrod	lynamic	Hydrod	ynamic	Hydrodynamic	
	Cost/Removal Analysis	Sepa	rator	Sepa	rator	Sepa	rator
		New	Net %	New	Net %	New	Net %
	TP (lb/yr)	8.0	36%	7.9	36%	7.7	36%
	TSS (lb/yr)	3,401	52%	3,355	51%	3,304	51%
	Volume (acre-feet/yr)	0.0	14%	0.0	14%	0.0	14%
ent	Number of BMP's	1		1		1	
reatma	BMP Size/Description	8	foot diameter	6	foot diameter	4	foot diameter
F	ВМР Туре	Hydrodynamic Separator		Hydrodynamic Separator		Hydrodynamic Separator	
	Materials/Labor/Design	\$45	,000	\$22,500		\$16	,500
	Promotion & Admin Costs	\$1,	752	\$1,752		\$1,752	
Cost	Probable Project Cost	\$46	,752	\$24,	,252	\$18	,252
	Annual O&M	\$4	20	\$4	20	\$4	20
	30-yr Cost/lb-TP	\$2	47	\$1	55	\$1	34
	30-yr Cost/1,000lb-TSS	\$5	82	\$366		\$311	

#### Hydrodynamic Separator (Hackman Ave. NE)

#### Project ID: ML-5 Grass Swale at East Moore Lake Park

Drainage Area – 0.64 acres

*Location* – Northwest corner of parking lot in East Moore Lake Park

**Property Ownership** – City of Fridley

**Description** – Portions of the main parking lot within East Moore Lake Park currently drain directly to Moore Lake without any stormwater treatment. The northern portion of the parking lot drains to the northwest into a paved swale that transports stormwater directly to the lake. Space is available to construct a vegetated swale that will treat stormwater before it enters the lake. Volume reduction and removal of TP and TSS are shown in the table below.

				Proje	ect ID	
	Cost/Removal Analysis	106' Gra	ss Swale			
	cost, nemoval Analysis	New trtmt	Net %			
	TP (lb/yr)	0.1	13%			
	TSS (lb/yr)	68	15%			
reatment	Volume (acre-feet/yr)	0.1	5%			
	Number of BMP's	1				
	BMP Size/Description	954	square feet			
L	ВМР Туре	Dry Swale				
	Materials/Labor/Design	\$6,	296			
ł	Promotion & Admin Costs	\$1,	168			 
Sosi	Probable Project Cost	\$7,	464			
	Annual O&M	\$5	84			
	30-yr Cost/lb-TP	\$8,	328			
	30-yr Cost/1,000lb-TSS	\$12	,247			

#### East Moore Lake Park Parking Lot Grass Swale

#### Project ID: ML-5 New Pond in East Moore Lake Park

Drainage Area – 101 acres

*Location* – South of the parking lot in East Moore Lake Park

Property Ownership – City of Fridley

**Description** – Stormwater runoff currently generated within catchment ML-5 is primarily routed through stormwater infrastructure (101 of 120 acres) and outlets to Moore Lake within East Moore Lake Park untreated. Some space is available within the park to develop a wet pond that would serve as pretreatment for the runoff prior to it entering the lake. Additional engineering and feasibility analysis is required before the project could move forward. Removal of TP and TSS are shown in the table below.

#### East Moore Lake Park New Pond

				Project ID				
	Cost/Removal Analysis	New Po Moore L	nd - East ake Park					
		New trtmt	Net %					
	TP (lb/yr)	15.5	27%					
ent	TSS (lb/yr)	5,972	36%					
	Volume (acre-feet/yr)	0.0	5%					
	Number of BMP's	1						
reatm	BMP Size/Description	522	cubic yards					
L	ВМР Туре	Wet Pond						
	Materials/Labor/Design	\$45	,337					
	Promotion & Admin Costs	\$5,	840					
Sos	Probable Project Cost	\$51	,177					
	Annual O&M	\$7	00					
	30-yr Cost/lb-TP	\$1	.55					
	30-yr Cost/1,000lb-TSS	\$4	03					

#### Proposed Site Image -



#### Additional Retrofit Considerations

If the retrofits proposed for this catchment are inadequate due to limited landowner participation or if additional treatment is desired, below is a list of other retrofits that could be used in this catchment. However, detailed model and cost estimates were not developed because the practices would be cost prohibitive or provide minimal pollutant reduction relative to the proposed retrofits for this catchment.

• Underground storage and reuse at Totino Grace High School

## **Catchment ML-6**

Existing Catchment	Summary
Acres	72
Dominant Land Cover	Residential
Parcels	236
TP (lbs/yr)	37.4
TSS (lbs/yr)	8,065
Volume (acre-feet/yr)	48.7

#### **CATCHMENT DESCRIPTION**

Catchment ML-6 contains a wide range of land uses including medium density residential, multi-family residential, office park, and North Park Elementary School. The catchment is located north of I-694 and east of Central Avenue North.

#### **EXISTING STORMWATER TREATMENT**

Existing stormwater treatment in catchment ML-6 consists of a large stormwater pond, a smaller pond

that treats a portion of the Holiday gas station, and street sweeping by the City of Fridley. The large stormwater pond located near the center of the catchment treats stormwater from approximately 75% (55 acres) of catchment ML-6. A biologically activated soil filtration unit (BASFU) located on the west side of the pond was originally designed and constructed as additional treatment, but it has been disconnected for many years and therefore provides no current stormwater treatment. The table below shows how existing treatment practices within catchment ML-6 affect the stormwater volume and pollutant loads entering East Moore Lake.

### **Existing Conditions**

	Existing Conditions	Base Loading	Treatment	Net Treatment %	Existing Loading		
	TP (lb/yr)	66.1	28.7	43%	37.4		
nt	TSS (lb/yr)	18,009	9,944.0	55%	8,065		
me	Volume (acre-feet/yr)	49.2	0.5	1%	48.7		
Treati	Number of BMP's	3					
	BMP Size/Description	Holiday pond, pond, street sweeping					



### **RETROFIT RECOMMENDATIONS**



#### Project ID: ML-6 Residential Rain Gardens Downstream of Existing Stormwater Pond

#### Drainage Area – 5.2 acres

Location – Downstream of stormwater treatment pond along Lynde Dr. NE

#### Property Ownership - Private

**Description** – The residential nature of this catchment makes it best suited to curb-cut rain gardens (see Appendix C for design options). Three ideal rain garden locations were identified (see map), though more may exist. The rain gardens downstream of the pond were modeled separately as to remove the treatment train effects associated with the existing stormwater treatment pond because stormwater treated by the proposed gardens is not currently treated by the pond. Generally, ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, a scenario with three rain gardens was analyzed to treat the residential land use. Catchment-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

#### Conceptual images -



Before/24 – 48 hours after rain



During rain

				Proje	ect ID	
	Cost/Removal Analysis	3 Curb- Gare	Cut Rain dens			
		New trtmt	Net %			
	TP (lb/yr)	1.9	46%			
reatment	TSS (lb/yr)	508	58%			
	Volume (acre-feet/yr)	1.2	3%			
	Number of BMP's	3				
	BMP Size/Description	750	square feet			
F	ВМР Туре	Complex Bioretention				
	Materials/Labor/Design	\$15	,000			
*	Promotion & Admin Costs	\$7,	045			
Cost	Probable Project Cost	\$22	,045			
	Annual O&M	\$2	25			
	30-yr Cost/lb-TP	\$5	605			
	30-yr Cost/1,000lb-TSS	\$1,	889			

#### **Residential Rain Gardens**

#### Project ID: ML-6 Residential Rain Gardens Upstream of Existing Stormwater Pond

#### Drainage Area – 41.3 acres

Location - Upstream of stormwater treatment pond

Property Ownership - Private

**Description** – The residential nature of this catchment makes it best suited to curb-cut rain gardens (see Appendix C for design options). Eleven ideal rain garden locations were identified (see map), though more may exist. The rain gardens upstream of the pond were modeled separately as to represent the treatment train effects associated with the existing stormwater treatment pond. Generally, ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, scenarios with 5 and 10 rain gardens were analyzed to treat the residential land use. Catchment-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

#### Conceptual images -



Before/24-48 hours after rain



During rain

Resid	ential Rain Gardens						
				Proje	ect ID		
	Cont/Domannel Angelasia	5 Curb-	Cut Rain	10 Curb-Cut Rain			
	Cost/Removal Analysis	Gard	dens	Gard	dens		
		New trtmt	Net %	New trtmt	Net %		
	TP (lb/yr)	1.8	46%	3.3	48%		
	TSS (lb/yr)	263	57%	494	58%		
	Volume (acre-feet/yr)	3.3	8%	5.8	13%		
reatment	Number of BMP's	5		1	10		
	BMP Size/Description	1,250	square feet	2,500	square feet		
1	ВМР Туре	Complex Bioretention		Complex Bi	oretention		
	Materials/Labor/Design	\$25	,000	\$50,	,000		
	Promotion & Admin Costs	\$9,	381	\$15,	,221		
Cost	Probable Project Cost	\$34	,381	\$65,	,221		
	Annual O&M	\$3	75	\$7	50		
-	30-yr Cost/lb-TP	\$8	45	\$8	86		
	30-yr Cost/1,000lb-TSS	\$5,	783	\$5,919			

#### Project ID: ML-6 Rain Garden at North Park Elementary School

Drainage Area - 5.0 acres

*Location* – South side of North Park Elementary School Campus

Property Ownership – North Park Elementary School

**Description** – Substantial open space exists on the south side of the North Park Elementary School campus along Regis Ln. NE (labeled stormwater redirect in the retrofit recommendations map). A curbcut rain garden could be installed to treat stormwater runoff from 5 acres of residential land use. Three garden sizes (500 sq. ft., 1,000 sq. ft., and 2,000 sq. ft.) were analyzed to treat the contributing drainage area. Catchment-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

Conceptual images -



Before/24-48 hours after rain North Park Elementary School Rain Garden



During rain

		Project ID							
	Cost/Removal Analysis	2,000 sq. ft. Curb- Cut Rain Gardens		1,000 sq. ft. Curb- Cut Rain Gardens		500 sq. ft. Curb-Cut Rain Gardens			
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %		
Treatment	TP (lb/yr)	1.7	46%	1.2	45%	0.7	44%		
	TSS (lb/yr)	282	57%	199	56%	118	56%		
	Volume (acre-feet/yr)	2.0	5%	1.5	4%	1.0	3%		
	Number of BMP's	1		1		1			
	BMP Size/Description	2,000	square feet	1,000	square feet	500	square feet		
	ВМР Туре	Simple Bioretention		Simple Bioretention		Simple Bioretention			
Cost	Materials/Labor/Design	\$24,476		\$13,176		\$7,526			
	Promotion & Admin Costs	\$2,920		\$2,920		\$2,920			
	Probable Project Cost	\$27,396		\$16,096		\$10,446			
	Annual O&M	\$75		\$75		\$75			
	30-yr Cost/lb-TP	\$581		\$510		\$605			
	30-yr Cost/1,000lb-TSS	\$3,504		\$3,073		\$3,586			

#### Project ID: ML-6 Permeable Asphalt at High Rise Residential along Lynde Dr. NE

Drainage Area – 0.6 acres or 0.4 acres

Location - Main parking lots of high rise residential buildings along Lynde Dr. NE

Property Ownership – Private

**Description** – Two apartment complexes are located on either side of Lynde Dr. NE between Hillwind Rd. NE and Polk St. NE. Both parking lots could be treated using permeable asphalt (see Appendix D for design options) prior to it entering the stormwater infrastructure. Permeable asphalt is well suited to this area because of the large amount of impervious surface. Volume reduction and removal of TP and TSS are shown in the table below.

#### **Project ID Permeable Asphalt Permeable Asphalt** Cost/Removal Analysis - 0.15 acre - 0.1 acre Net % TP (lb/yr) 44% 44% 0.5 0.3 TSS (lb/yr) 300 57% 200 56% Volume (acre-feet/yr) 0.8 3% 0.6 2% Treatment Number of BMP's 1 1 square square **BMP Size/Description** 4,356 6,534 feet feet Permeable Asphalt Permeable Asphalt Materials/Labor/Design \$67,216 \$45,436 Promotion & Admin \$1,752 \$1,752 \$68,968 \$47,188 Annual O&M \$150 \$100 30-yr Cost/lb-TP \$4,898 \$5,577 30-yr Cost/1,000lb-TSS \$8,164 \$8,366

#### High Rise Residential Permeable Asphalt

#### Project ID: ML-6 Hydrodynamic Separator (Polk St. NE)

Drainage Area – 5.2 acres

Location – Near intersection of Polk St. NE and Lynde Dr. NE

Property Ownership – City of Fridley

**Description** – The confluence of multiple stormwater lines within catchment ML-6 near the intersection of Polk St. NE and Lynde Dr. NE is a potential site for a hydrodynamic separator (see Appendix D for additional information). The structural best management practice uses hydrodynamic separation to remove particulate pollutants from stormwater and could be installed in place of an existing catch basin. Scenarios with 4, 6, and 8 foot diameter devices were analyzed to treat the residential land use. Removal of TP and TSS could be increased to the levels shown in the following table.

#### Project ID Hydrodynamic Hydrodynamic Hydrodynamic Cost/Removal Analysis Separator Separator Separator Net % Net % Net % TP (lb/yr) 0.5 44% 0.4 44% 0.3 44% TSS (lb/yr) 56% 140 56% 176 56% 101 Volume (acre-feet/yr) 0.0 0.0 1% 1% 0.0 1% Number of BMP's 're atment 1 1 1 foot foot foot **BMP Size/Description** 8 6 4 diameter diameter diameter Hydrodynamic Hydrodynamic Hydrodynamic **BMP Type** Separator Separator Separator Materials/Labor/Design \$45,000 \$22,500 \$16,500 Promotion & Admin \$1,752 \$1,752 \$1,752 Cost **Probable Project Cost** \$46,752 \$24,252 \$18,252 Annual O&M \$420 \$420 \$420 30-yr Cost/lb-TP \$3,957 \$3,428 \$3,071 30-yr Cost/1,000lb-TSS \$11,241 \$8,774 \$10,182

#### Hydrodynamic Separator (Polk St. NE)

#### Project ID: ML-6 Iron enhanced sand filter for existing pond

#### Drainage Area - 54.6 acres

*Location* – West side of wet detention pond located in the center of catchment ML-6 *Property Ownership* – City of Fridley

**Description** – Retrofitting the existing wet pond with an iron enhanced sand filter along the eastern edge of the pond would increase the pond's efficiency at removing dissolved phosphorus (Erickson & Gulliver 2010). A significant percentage of phosphorus in stormwater is dissolved (30% - 45%).

The iron enhanced sand filter would be installed at an elevation slightly above the normal water level of the pond so that following a storm event the increase in depth of the pond would be first diverted to the iron enhanced sand filter. The filter would have drain tile installed along the base of the trench and would outlet downstream of the current pond outlet (see schematic below). Large storm events that overwhelm the iron enhanced sand filter's capacity would exit the pond via the existing outlet.

Based on available space and the relatively large contributing drainage area, a 230 foot long by 100 foot wide by 2 foot deep filter with one foot of live storage above the iron enhanced sand filter was modeled. Networkwide volume and pollutant removal are shown in the table below. Please note that the iron enhanced sand filter would need to be an engineered project.



	Cost/Removal Analysis	Project ID						
		IESF						
		New trtmt	Net %					
Treatment	TP (lb/yr)	4.3	50%					
	TSS (lb/yr)	0	55%					
	Volume (acre-feet/yr)	0.0	1%					
	Number of BMP's	1						
	BMP Size/Description	230	linear feet					
	ВМР Туре	Perimeter Iron Enhanced Sand Filter						
Cost	Materials/Labor/Design	\$46,500						
	Promotion & Admin Costs	\$1,752						
	Probable Project Cost	\$48,252						
	Annual O&M	\$1,550						
	30-yr Cost/lb-TP	\$735						
	30-yr Cost/1,000lb-TSS	N/A						

#### Iron Enhanced Sand Filter
# **Section 2: North Network**

Existing Network Summary							
Acres	375						
	Residential,						
Dominant Land Cover	Park,						
	Commercial						
Parcels	922						
TP (lbs/yr)	211.9						
TSS (lbs/yr)	38,909						
Volume (acre-feet/yr)	223.5						

### NETWORK CATCHMENTS

Catchment ID	Page
ML-2	36
ML-3	41
ML-4	47

#### **EXISTING NETWORK TREATMENT**

The image to the right represents a simplified flow network for the catchments within the North Network connected to East Moore Lake. Stormwater infrastructure throughout the connected catchments consists of pipes and open channel ditches. Existing stormwater treatment consists of several treatment ponds and street sweeping. The combination of these existing treatment practices results in a 36% TP reduction relative to base conditions. Connected catchments will only have network level reductions reported in the catchment profile because those reductions most accurately reflect the benefit to Moore Lake and the true cost effectiveness of each project.





# **Catchment ML-2**

Existing Catchment Summary*							
Acres	58						
Dominant Land Cover	Residential, Park						
Parcels	130						
TP (lbs/yr)	42.5						
TSS (lbs/yr)	10,090						
Volume (acre-feet/yr)	24.5						

\*Excludes network-wide treatment practices

#### CATCHMENT DESCRIPTION

Catchment ML-2 consists of five different land use types (freeway, medium density residential, open space, park, and open water). The catchment drains directly to East Moore Lake and is positioned on the northeast side of the lake. Residential land use is approximately 70% of the total area, while Moore Lake Park represents 24% of the area.



### **EXISTING STORMWATER TREATMENT**

In addition to street sweeping by the City of Fridley, the majority of stormwater generated from the residential land use in catchment ML-2 passes through a treatment pond located on the north side of the lake. The pond is undersized because it receives stormwater from catchments ML-2, ML-3, and ML-4 (approximately 374 acres). Most of Moore Lake Park enters the lake untreated. The table below shows the network-wide base and existing conditions. The network-wide table shows how existing treatment practices within catchment ML-2 affect the stormwater volume and pollutant load in East Moore Lake.

#### Network-Wide Existing Conditions

	Existing Conditions	Base Loading	Treatment	Net Treatment	Existing Loading			
	TP (lb/yr)	332.3	120.4	36%	211.9			
nt	TSS (lb/yr)	85,347	46,438.0	54%	38,909			
me	Volume (acre-feet/yr)	226.1	2.6	1%	223.5			
eat	Number of BMP's	9						
Tre	BMP Size/Description	Ponds, rain garden, exfiltration pipe, street sweeping						

## **RETROFIT RECOMMENDATIONS**



#### Project ID: ML-2 Residential Rain Gardens

#### Drainage Area - 28 acres

Location – East of Old Central Ave. NE, predominantly along Woody Ln. NE

#### Property Ownership - Private

**Description** – The residential land use within this catchment is well suited for curb-cut rain gardens (see Appendix C for design options). Ten ideal rain garden locations were identified (see map), though more exist. Generally ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, scenarios with 3, 5, and 10 rain gardens were analyzed to treat the residential land use. Network-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

#### Conceptual images -



Before/24-48 hours after rain



During rain

#### Rain Gardens Treating Single Family, Medium Density Residential Land Use (Network-Wide)

			Project ID					
	Cost/Removal Analysis	3 - Curb-Cut Rain Gardens		5 - Curb-Cut Rain Gardens		10 - Curb-Cut Rain Gardens		
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %	
	TP (lb/yr)	1.9	37%	3.0	37%	5.0	38%	
	TSS (lb/yr)	422	55%	657	55%	1,127	56%	
reatment	Volume (acre-feet/yr)	2.0	2%	3.0	2%	4.9	3%	
	Number of BMP's	3		5		10		
	BMP Size/Description	750	square feet	1,250	square feet	2,500	square feet	
L	ВМР Туре	Complex Bioretention		Complex Bioretention		Complex Bioretention		
	Materials/Labor/Design	\$15	,000	\$25	,000	\$50	,000	
-	Promotion & Admin Costs	\$7,	045	\$9,381		\$15,221		
Sosi	Probable Project Cost	\$22	,045	\$34	,381	\$65	,221	
	Annual O&M	\$2	.25	\$375		\$7	50	
	30-yr Cost/lb-TP	\$5	05	\$5	07	\$5	85	
	30-yr Cost/1,000lb-TSS	\$2,	274	\$2,315		\$2,595		

### Project ID: ML-2 Hydrodynamic Separator (Old Central Ave. NE)

Drainage Area – 17 acres

*Location* – Old Central Ave. NE near East Moore Lake Park

**Property Ownership** – City of Fridley

**Description** – The confluence of multiple stormwater lines within catchment ML-2 along Old Central Ave. NE near East Moore Lake Park is a potential site for a hydrodynamic separator (see Appendix D for additional information). The structural best management practice uses hydrodynamic separation to remove particulate pollutants from stormwater and could be installed in place of an existing catch basin. Scenarios with 4, 6, and 8 foot diameter devices were analyzed to treat the residential land use. Volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

#### Hydrodynamic Separator

		Project ID						
	Cost/Removal Analysis	Hydrodynamic Separator		Hydrod Sepa	ynamic rator	Hydrodynamic Separator		
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %	
	TP (lb/yr)	0.4	36%	0.4	36%	0.3	36%	
	TSS (lb/yr)	130	55%	109	55%	71	54%	
	Volume (acre-feet/yr)	0.0	1%	0.0	1%	0.0	1%	
reatment	Number of BMP's	1		1		1		
	BMP Size/Description	8	foot diameter	6	foot diameter	4	foot diameter	
T	ВМР Туре	Hydrodynamic Separator		Hydrodynamic Separator		Hydrodynamic Separator		
	Materials/Labor/Design	\$45,000		\$22,500		\$16,500		
	Promotion & Admin Costs	\$1,	752	\$1,752		\$1,752		
Sosi	Probable Project Cost	\$46	,752	\$24,	,252	\$18	,252	
	Annual O&M	\$4	20	\$4	20	\$4	20	
	30-yr Cost/lb-TP	\$4,	946	\$3,	071	\$3,	428	
	30-yr Cost/1,000lb-TSS	\$15,218		\$11,270		\$14,485		

#### Project ID: ML-2 New Pond

Drainage Area – 17 acres

Location - West of Old Central Ave. NE near Woody Ln. NE

**Property Ownership** – City of Fridley

**Description** – Substantial open space exists west of Old Central Ave. NE near Woody Ln. NE. The property is owned by the City of Fridley and presents an opportunity for a new stormwater wet pond. Redirection of existing stormwater lines to the new pond is an option to provide additional treatment to the residential land use within catchment ML-2.

Analysis was completed for excavating the pond to provide four feet of ponding. Due to the existing topography within the proposed pond area, significant excavation is required to achieve the desired ponding depth. The network-wide modeled annual TP removal is only 2.4 pounds because most of the phosphorus is already treated by the existing pond on the north side of East Moore Lake. Additional engineering and feasibility analysis is required before the project could move forward. Network-wide removal of TSS and TP could be increased to the levels shown in the following table.

			Project ID				
	Cost/Removal Analysis	New Por Moore L	id in East ake Park				
		New trtmt	Net %				
	TP (lb/yr)	2.4	37%				
	TSS (lb/yr)	701	55%				
nt	Volume (acre-feet/yr)	0.0	1%				
Treatme	Number of BMP's	1					
	BMP Size/Description	8,878	cubic yards				
	ВМР Туре	Wet Pond					
	Materials/Labor/Design	\$217	7,404				
	Promotion & Admin Costs	\$5,840					
Sosi	Probable Project Cost	\$223	8,244				
	Annual O&M	\$4	50				
	30-yr Cost/lb-TP	\$3,	288				
	30-yr Cost/1,000lb-TSS	\$11	,257				

#### **New Pond**



Moore Lake Stormwater Retrofit Analysis

# **Catchment ML-3**

Existing Catchment Summary*							
Acres	186						
Dominant Land Cover	Residential, Commercial						
Parcels	374						
TP (lbs/yr)	146.2						
TSS (lbs/yr)	42,912						
Volume (acre-feet/yr)	123.4						

\*Excludes network-wide treatment practices

#### **CATCHMENT DESCRIPTION**

Catchment ML-3 consists of 7 different land use types (freeway, office park, open space, park, commercial, high-rise residential and medium density residential). The catchment drains into the northeast corner of East Moore Lake. Medium density residential makes up 66% of the catchment and strip commercial makes up another 14%.



#### **EXISTING STORMWATER TREATMENT**

Existing stormwater treatment in ML-3 consists of multiple treatment ponds and street sweeping by the City of Fridley. Site specific stormwater treatment ponds exist near Lifetime Fitness and the Landmark of Fridley. In addition, a pond located in Creekridge Park provides treatment of stormwater for approximately nine acres of residential and park land use. An exfiltration pipe to promote infiltration exists downstream of the Creekridge Park pond. However, its condition is unknown. All water from catchment ML-3 is treated by the pond on the north side of East Moore Lake located in catchment ML-2, though it is undersized for the contributing drainage area. The network-wide table shows how existing treatment practices within catchment ML-3 affect the stormwater volume and pollutant load in East Moore Lake.

#### **Network-Wide Existing Conditions**

	Existing Conditions	Base Loading	Treatment	Net Treatment %	Existing Loading				
	TP (lb/yr)	332.3	120.4	36%	211.9				
nt	TSS (lb/yr)	85,347	46,438.0	54%	38,909				
me	Volume (acre-feet/yr)	226.1	2.6	1%	223.5				
eat	Number of BMP's	9							
Tre	BMP Size/Description	Ponds, rain garden, exfiltration pipe, street sweeping							

# **RETROFIT RECOMMENDATIONS**



### Project ID: ML-3 Residential Rain Gardens

#### Drainage Area – 57 acres

Location - Throughout residential land use within catchment ML-3

### Property Ownership - Private

**Description** – The residential land use within this catchment is well suited for curb-cut rain gardens (see Appendix C for design options). Seventeen ideal rain garden locations were identified (see map), though more may exist. Generally ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, scenarios with 5, 10, and 15 rain gardens were analyzed to treat the residential land use. Network-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

### Conceptual images -



Before/24-48 hours after rain



During rain

#### Rain Gardens Treating Single Family, Medium Density Residential Land Use (Network-Wide)

		Project ID						
	Existing Conditions	5 - Curb- Gare	5 - Curb-Cut Rain Gardens		10 - Curb-Cut Rain Gardens		15 - Curb-Cut Rain Gardens	
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %	
	TP (lb/yr)	3.3	37%	5.9	38%	8.1	39%	
	TSS (lb/yr)	721	55%	1,317	56%	1,822	57%	
ent	Volume (acre-feet/yr)	3.4	3%	6.1	4%	8.2	5%	
	Number of BMP's	5		10		15		
reatm	BMP Size/Description	1,250	square feet	2,500	square feet	3,750	square feet	
L	ВМР Туре	Complex Bioretention		Complex Bioretention		Complex Bioretention		
	Materials/Labor/Design	\$25	,000	\$50,	,000	\$75	,000	
Ļ	Promotion & Admin Costs	\$9,	381	\$15,221		\$21,061		
Soc	Probable Project Cost	\$34	,381	\$65,	,221	\$96	,061	
	Annual O&M	\$3	75	\$7	50	\$1,	125	
	30-yr Cost/lb-TP	\$4	61	\$4	96	\$5	34	
	30-yr Cost/1,000lb-TSS	\$2,	110	\$2,220		\$2,375		

#### Project ID: ML-3 New Pond East of Old Central Ave. NE

#### Drainage Area – 28 acres

Location – East of Old Central Ave. NE and south of Mississippi St. NE

Property Ownership – Private

**Description** – Substantial open space exists east of Old Central Ave. NE just south of Mississippi St. NE. The property is privately owned, but presents an opportunity for a new stormwater wet pond. Redirection of an existing stormwater line to the new pond is an option to provide additional treatment to the residential land use within catchment ML-3.

Analysis was completed for excavating the pond to provide six feet of ponding. The tax value of the property (\$47,600) was included in the cost estimate because the property is currently privately owned. The network-wide modeled annual TP removal is only 2.0 pounds because most of the phosphorus is already treated by the existing pond on the north side of East Moore Lake. Additional engineering and feasibility analysis is required before the project could move forward. Network-wide removal of TSS and TP could be increased to the levels shown in the following table.

			Project ID				
	Existing Conditions	New Pon Old Cen	d East of tral Ave.				
		New trtmt	Net %				
	TP (lb/yr)	2.0	37%				
	TSS (lb/yr)	657	55%				
nt	Volume (acre-feet/yr)	0.0	1%				
Treatme	Number of BMP's	1					
	BMP Size/Description	3,647	cubic yards				
	ВМР Туре	Wet Pond					
	Materials/Labor/Design	\$158	3,708				
	Promotion & Admin Costs	\$5,840					
Jost	Probable Project Cost	\$164	1,548				
	Annual O&M	\$4	50				
	30-yr Cost/lb-TP	\$2,	967				
	30-yr Cost/1,000lb-TSS	\$9,	033				

#### **New Pond**



Moore Lake Stormwater Retrofit Analysis

### Project ID: ML-3 New Pond North of Rice Creek Rd.

#### Drainage Area – 131 acres

Location - North of Rice Creek Rd. and east of Old Central Ave. NE

**Property Ownership** – City of Fridley

**Description** – City owned lots exist on the north side of Rice Creek Rd. and present an opportunity for a new stormwater wet pond that would treat runoff from portions of ML-3 and all of ML-4 if the existing ditch was routed into the proposed pond.

Analysis was completed for excavating the pond to provide six feet of ponding. The networkwide modeled annual TP removal is only 0.6 pounds because most of the phosphorus is already treated by the existing pond on the north side of East Moore Lake. In addition, Harris Pond provides significant treatment of all land use within ML-4. Additional engineering and feasibility analysis is required before the project could move forward. Network-wide removal of TSS and TP could be increased to the levels shown in the following table.

				Pro	ject ID	
	Existing Conditions	New Pone Rice Cr	d North of eek Rd.			
		New trtmt	Net %			
	TP (lb/yr)	0.6	36%			
	TSS (lb/yr)	251	55%			
ent	Volume (acre-feet/yr)	0.0	1%			
Treatm	Number of BMP's	1				
	BMP Size/Description	11,037	cubic yards			
	ВМР Туре	Wet Pond				
	Materials/Labor/Design	\$234,608				
	Promotion & Admin Costs	\$5,840				
Sosi	Probable Project Cost	\$240	),448			
	Annual O&M	\$4	50			
	30-yr Cost/lb-TP	\$14	,108			
	30-yr Cost/1,000lb-TSS	\$33	,725			

#### **New Pond**



#### Project ID: ML-3 Pond Modification in Creekridge Park

Drainage Area – 9 acres

*Location* – Existing pond within Creekridge Park

**Property Ownership** – City of Fridley

**Description** – The existing stormwater pond within Creekridge Park currently provides little treatment because of a shallow ponding depth and small size. Some potential exists for additional excavation to increase ponding depth and overall size.

Analysis was completed for excavating the pond to provide three feet of ponding and expand the footprint of the pond. The network-wide modeled annual TP removal is only 2.0 pounds because most of the phosphorus is already treated by the existing pond on the north side of East Moore Lake. Additional engineering and feasibility analysis is required before the project could move forward. Network-wide removal of TSS and TP could be increased to the levels shown in the following table.

#### **Pond Modification**

			Project ID				
	Existing Conditions	Pond Mo in Creekr	dification idge Park				
		New trtmt	Net %				
	TP (lb/yr)	2.0	37%				
	TSS (lb/yr)	677	55%				
ent	Volume (acre-feet/yr)	0.0	1%				
ţ	Number of BMP's	1					
Trea	BMP Size/Description	861	cubic yards				
	ВМР Туре	Wet Pond					
	Materials/Labor/Design	\$26,269					
	Promotion & Admin Costs	\$5,840					
Sost	Probable Project Cost	\$32,109					
Ŭ	Annual O&M	\$4	50				
	30-yr Cost/lb-TP	\$7	/60				
	30-yr Cost/1,000lb-TSS	\$2,246					



Moore Lake Stormwater Retrofit Analysis

# **Catchment ML-4**

Existing Catchment Summary*						
Acres	131					
Dominant Land Cover	Residential					
Parcels	418					
TP (lbs/yr)	64.6					
TSS (lbs/yr)	2,490					
Volume (acre-feet/yr)	75.5					

\*Excludes network-wide treatment practices

#### CATCHMENT DESCRIPTION

Catchment ML-4 is predominantly medium density residential land use (88%). Other land uses present in the catchment are park, office park, and open water. The majority of the catchment lies in the city of Fridley, but the eastern third crosses into New Brighton.



#### **EXISTING STORMWATER TREATMENT**

All stormwater from catchment ML-4 passes through Harris Pond, which essentially functions as a large stormwater treatment pond. The City of Fridley also conducts street sweeping throughout catchment ML-4. Finally, all water from catchment ML-4 is also treated by the pond on the north side of East Moore Lake located in catchment ML-2, though it is undersized for the contributing drainage area. The network-wide table below shows how existing treatment practices within catchment ML-4 affect the stormwater volume and pollutant load to East Moore Lake.

#### Network-Wide Existing Conditions

	Existing Conditions	Base Loading	Treatment	Net Treatment %	Existing Loading		
	TP (lb/yr)	332.3	120.4	36%	211.9		
nt	TSS (lb/yr)	85,347	46,438.0	54%	38,909		
me	Volume (acre-feet/yr)	226.1	2.6	1%	223.5		
eati	Number of BMP's	9					
μ	BMP Size/Description	Ponds, rain garden, exfiltration pipe, street sweeping					

# **RETROFIT RECOMMENDATIONS**



### Project ID: ML-4 Residential Rain Gardens

#### Drainage Area - 115.6 acres

Location - Throughout the residential land use in ML-4

### Property Ownership – Private

**Description** – The residential nature of this catchment makes it best suited to curb-cut rain gardens (see Appendix C for design options). Eighty one ideal rain garden locations were identified (see map), though more may exist. Generally, ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, scenarios with 10, 20, and 30 rain gardens were analyzed to treat the residential land use. Catchment-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

#### Conceptual images -

**Residential Rain Gardens** 



Before/24-48 hours after rain



During rain

		Project ID						
	Cost/Removal Analysis	10 - Curb-Cut Rain Gardens		20 - Curb-Cut Rain Gardens		30 - Curb-Cut Rain Gardens		
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %	
	TP (lb/yr)	4.6	38%	8.4	39%	11.5	40%	
	TSS (lb/yr)	862	55%	1,605	56%	2,200	57%	
	Volume (acre-feet/yr)	6.9	4%	12.2	7%	16.4	8%	
Treatment	Number of BMP's	10		20		30		
	BMP Size/Description	2,500	square feet	5,000	square feet	7,500	square feet	
	ВМР Туре	Complex Bioretention		Complex Bioretention		Complex Bioretention		
	Materials/Labor/Design	\$50	,000	\$100,000		\$150,000		
	Promotion & Admin Costs	\$15,221		\$26,901		\$38,581		
SoS	Probable Project Cost	\$65	,221	\$126,901		\$188,581		
	Annual O&M	\$7	'50	\$1,500		\$2,250		
	30-yr Cost/lb-TP	\$6	36	\$682		\$7	42	
	30-yr Cost/1,000lb-TSS	\$3,392		\$3,570		\$3,880		

# **Section 3: Disconnected Catchments**

Existing Network Summary						
Acres	277					
Dominant Land Cover	Residential					
Parcels	1,251					
TP (lbs/yr)	N/A					
TSS (lbs/yr)	N/A					
Volume (acre-feet/yr)	N/A					

Catchment ID	Page
ML-7	51
ML-8	53
ML-9	56
ML-10	58



### **EXISTING TREATMENT**

Catchments in this section were found to have no connection to Moore Lake. Therefore, no formal analyses were completed for the included catchments, with the exception of ML-8 because Innsbruck Nature Center was deemed a resource of interest.

# **Catchment ML-7**

Existing Catchment Summary						
Acres	88					
Dominant Land Cover	Residential,					
Dominant Earld Cover	Open Space					
Parcels	264					
TP (lbs/yr)	N/A					
TSS (lbs/yr)	N/A					
Volume (acre-feet/yr)	N/A					

#### **CATCHMENT DESCRIPTION**

Catchment ML-7 is comprised primarily of medium density, residential land use (80%). There is also a sizeable area of open space on the far western lobe of the catchment. Other land uses present include school, multi-family residential, and open water. Approximately half of the Islamic Center of Minnesota campus is located in catchment ML-7.



#### **EXISTING STORMWATER TREATMENT**

All stormwater in this catchment is directed to isolated stormwater ponds or wetland areas with no regular connection to Moore Lake. However, excessive precipitation has resulted in documented overflow near the far western lobe into catchment ML-2.

Stormwater east of Stinson Blvd. is conveyed north to existing ponds on the north side of 18<sup>th</sup> St. NW. The west side of Stinson Blvd. drains to the wetland complex behind the Islamic Center of Minnesota as well as several isolated low lying areas.

#### **RETROFIT RECOMMENDATIONS**

Due to the lack of connection to Moore Lake, no retrofits were recommended.

## **RETROFIT RECOMMENDATIONS**



Moore Lake Stormwater Retrofit Analysis

# **Catchment ML-8**

Existing Catchment Summary						
Acres	54					
Dominant Land Cover	Residential, Open Space					
Parcels	695					
TP (lbs/yr)	33.5					
TSS (lbs/yr)	8,180					
Volume (acre-feet/yr)	28.8					

#### **CATCHMENT DESCRIPTION**

Catchment ML-8 consists of a wide variety of land uses. The western half is primarily medium density residential while the eastern half is a mix of multifamily residential, hi-rise residential, open space, and parkland. The catchment includes portions of Innsbruck Nature Center.



### **EXISTING STORMWATER TREATMENT**

Innsbruck Nature Center was identified as a resource of interest. Therefore, modeling and retrofit recommendations for this catchment were completed even though it is disconnected from Moore Lake. Nevertheless, the stormwater ultimately enters the wetland complex located on the Innsbruck Nature Center campus and additional treatment was desired. Existing stormwater treatment practices within catchment ML-8 primarily consist of outlets in wetland areas and street sweeping by the City of Fridley. The table below shows how existing treatment practices within catchment ML-8 affect the stormwater volume and pollutant load to the Innsbruck Nature Center.

#### **Existing Conditions**

	Existing Conditions	Base Loading	Treatment	Net Treatment %	Existing Loading		
nt	TP (lb/yr)	39.1	5.6	14%	33.5		
	TSS (lb/yr)	10,305	2,125.0	21%	8,180		
me	Volume (acre-feet/yr)	28.8	0.0	0%	28.8		
eat	Number of BMP's	2					
л	BMP Size/Description	Pond, street sweeping					

# **RETROFIT RECOMMENDATIONS**



Moore Lake Stormwater Retrofit Analysis

### Project ID: ML-8 Residential Rain Gardens

#### Drainage Area – 9.6 acres

Location – Throughout catchment ML-8 in residential land use

Property Ownership - Private

**Description** – The residential nature of this catchment makes it best suited to curb-cut rain gardens (see Appendix C for design options). Six ideal rain garden locations were identified (see map), though more may exist. Generally, ideal rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical landowner participation rates, scenarios with 1, 3, and 5 rain gardens were analyzed to treat the residential land use. Catchment-wide volume reduction and removal of TP and TSS could be increased to the levels shown in the following table.

Conceptual images -



Before/24-48 hours after rain



During rain

#### Curb-Cut Rain Gardens Treating Residential Land Use

		Project ID							
	Cost/Removal Analysis	1 - Curb-Cut Rain Garden		3 - Curb-Cut Rain Garden		5 - Curb-Cut Rain Garden			
		New trtmt	Net %	New trtmt	Net %	New trtmt	Net %		
	TP (lb/yr)	0.4	15%	1.0	17%	1.5	18%		
	TSS (lb/yr)	58	21%	152	22%	226	23%		
	Volume (acre-feet/yr)	0.7	2%	1.5	5%	2.1	7%		
ent	Number of BMP's		1	3		5			
reatm	BMP Size/Description	250	square feet	750	square feet	1,250	square feet		
п	ВМР Туре	Complex Bioretention		Complex Bioretention		Complex Bioretention			
	Materials/Labor/Design	\$5,000		\$15,000		\$25,000			
-	Promotion & Admin Costs	\$4,709		\$7,045		\$9,381			
sog	Probable Project Cost	\$9,	709	\$22,045		\$34,381			
	Annual O&M	\$7	75	\$225		\$375			
	30-yr Cost/lb-TP	\$9	97	\$960		\$1,	014		
	30-yr Cost/1,000lb-TSS	\$6,873		\$6,315		\$6,730			

# **Catchment ML-9**

Existing Catchment Summary						
Acres	53					
Dominant Land Cover	Residential, Open Space					
Parcels	192					
TP (lbs/yr)	N/A					
TSS (lbs/yr)	N/A					
Volume (acre-feet/yr)	N/A					

### **CATCHMENT DESCRIPTION**

Nearly half of catchment ML-9 is composed of medium density residential and another quarter is multi-family residential. The last quarter is divided between freeway, open space, and open water land uses. ML-9 lies just north of I-694 on the eastern edge of the Moore Lake subwatershed.



#### **EXISTING STORMWATER TREATMENT**

All stormwater in this catchment is directed to isolated stormwater ponds, wetland areas, or Farr Lake with no connection to Moore Lake. The western most portion of the catchment flows to a stormwater pond that is connected via pipe to another treatment pond, which ultimately has a pipe connection to Farr Lake. Stormwater from the eastern side of the catchment enters Farr Lake directly.

#### **RETROFIT RECOMMENDATIONS**

Due to the lack of connection to Moore Lake, no retrofits were recommended.

# **RETROFIT RECOMMENDATIONS**



# **Catchment ML-10**

Existing Catchment Summary						
Acres	82					
Dominant Land Cover	Freeway, Residential					
Parcels	100					
TP (lbs/yr)	N/A					
TSS (lbs/yr)	N/A					
Volume (acre-feet/yr)	N/A					

### **CATCHMENT DESCRIPTION**

Catchment ML-10 is mostly comprised of I-694 but also contains part of a medium density residential neighborhood south of the freeway and a portion of the Menards store campus in the southwest corner.



### **EXISTING STORMWATER TREATMENT**

All stormwater in this catchment is directed to isolated stormwater ponds or wetland areas with no connection to Moore Lake. The residential area near Skywood Ln. NE drains north to a large, isolated depression on the south side of I-694. Stormwater from the high-rise residential complex and Menards is directed west toward the treatment ponds at the I-694 and Central Ave. NE exchange.

#### **RETROFIT RECOMMENDATIONS**

Due to the lack of connection to Moore Lake, no retrofits were recommended.

## **RETROFIT RECOMMENDATIONS**



# **Retrofit Ranking**

The tables on the next pages summarize potential projects. Potential projects are organized from most cost effective to least, based on cost per pound of total phosphorus removed. Installation of projects in series will result in lower total treatment than the simple sum of treatment across the individual projects due to treatment train effects. Reported treatment levels are dependent upon optimal siting and sizing. More detail about each project can be found in the catchment profile pages of this report. Projects that were deemed unfeasible due to prohibitive size, number, or were too expensive to justify installation are not included in the tables on the next pages.

Catchments 1 through 6: Summary of preferred stormwater retrofit opportunities ranked by cost-effectiveness with respect to total phosphorus (TP) reduction. Volume and total suspended solids (TSS) reductions are also shown. For more information on each project refer to the catchment profile

pages i	in this repo	irt.								
Project Rank	Catchment ID	Retrofit Type (refer to catchment profile pages for additional detail)	Projects Identified	TP Reduction (lb/yr)	TSS Reduction (Ib/yr)	Volume Reduction (ac-ft/yr)	Total Project Cost	Estimated Annual Operations & Maintenance (2013 Dollars)	Estimated cost/ 1,000lb-TSS (30-year)	Estimated cost/ lb-TP (30-year)
1	ML-5	New Wet Pond (East Moore Lake Park)	1	15.5	5972	0.0	\$51,177	\$700	\$403	\$155
2	ML-5	Residential Rain Gardens	5 - 20	5.0 - 15.1	1,425 - 4,221	3.5 - 10.3	\$38,761 - \$144,421	\$375 - \$1,500	\$1,170 - \$1,496	\$333 - \$418
3	ML-1	Residential Rain Gardens	1 - 5	1.1 - 4.1	307 - 1,158	0.8 - 2.8	\$10,585 - \$38,761	\$75 - \$375	\$1,346 - \$1,440	\$388 - \$407
4	WL-3	Residential Rain Gardens	5 - 15	3.3 - 8.1	721 - 1,822	3.4 - 8.2	\$34,381 - \$96,061	\$375 - \$1,125	\$2,110 - \$2,375	\$461 - \$534
5	9-1W	Residential Rain Gardens (downstream of pond)	3	1.9	508	1.2	\$22,045	\$225	\$1,889	\$505
9	ML-2	Residential Rain Gardens	3 - 10	1.9 - 5.0	422 - 1,127	2.0 - 4.9	\$22,045 - \$65,221	\$225 - \$750	\$2,274 - \$2,595	\$505 - \$585
7	9-TW	Rain Garden (Elementary School)	1	0.7 - 1.7	118 - 282	1.0 - 2.0	\$10,446 - \$27,396	\$75	\$3,073 - \$3,586	\$510 - \$605
8	ML-4	Residential Rain Gardens	10 - 30	4.6 - 11.5	862 - 2,200	6.9 - 16.4	\$65,221 - \$188,581	\$750 - \$2,250	<b>\$3,392 - \$3,880</b>	Ş636 - Ş742
6	9-1W	Iron Enhanced Sand Filter Pond Modification	1	4.3	0	0.0	\$48,252	\$1,550	N/A	\$735
10	ML-3	Pond Modification (Creekridge Park)	1	2.0	677	0.0	\$32,109	\$450	\$2,246	\$760
11	9-TW	Residential Rain Gardens (upstream of pond)	5 - 10	1.8 - 3.3	263 - 494	3.3 - 5.8	\$34,381 - \$65,221	\$375 - \$750	\$5,783 - \$5,919	\$845 - \$886
12	ML-1	Hydrodynamic Device (59th)	1	0.5 - 0.9	224 - 392	0.0	\$18,252 - \$46,752	\$420	\$3,900 - \$5,047	\$1,755 - \$2,198
13	ML-1	Grass Swale (Church)	1	0.4	181	0.2	\$14,896	\$584	\$5,970	\$2,701
14	ML-3	New Wet Pond (Old Central Ave.)	1	2.0	657	0.0	\$164,548	\$450	\$9,033	\$2,967
15	ML-1	Hydrodynamic Device (58th)	1	0.3 - 0.5	113 - 199	0.0	\$18,252 - \$46,752	\$420	\$7,824 - \$9,942	\$3,071 - \$3,957
Pollution re	duction benefits a	nd costs can not be summed with other projects in the san	ne catchment be	cause they are a	Ilternative option	is for treating the	same source area.			

Directly Connected Catchment

North Network

Moore Lake Stormwater Retrofit Analysis

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Proje Ranl	ct Catchment	Retrofit Type (refer to catchment profile pages for additional detail)	Projects Identified	TP Reduction (lb/yr)	TSS Reduction (lb/yr)	Volume Reduction (ac-ft/yr)	Total Project Cost	Estimated Annual Operations & Maintenance (2013 Dollars)	Estimated cost/ 1,000lb-TSS (30-year)	Estimated cost/ lb-TP (30-year)
16	9-TW	Hydrodynamic Device (Polk)	1	0.3 - 05	101 - 176	0.0	\$18,252 - \$46,752	\$420	\$8,774 - \$11,241	\$3,071 - \$3,957
17	ML-2	Hydrodynamic Device	1	0.3 - 0.4	71 - 130	0.0	\$18,252 - \$46,752	\$420	\$11,270 - \$15,218	\$3,071 - \$4,946
18	ML-2	New Wet Pond (East Moore Lake Park)	1	2.4	701	0.0	\$223,244	\$450	\$11,257	\$3,288
19	ML-1	Permeable Asphalt (Fridley High School)	1	1.9	632	1.7	\$208,662	\$476	\$11,758	\$3,911
20	ML-5	Permeable Asphalt (Totino Grace)	1	1.7	1,001	2.8	\$219,552	\$501	\$7,812	\$4,600
21	ML-1	Permeable Asphalt (Church)	1	1.0	615	1.7	\$136,752	\$311	\$7,917	\$4,869
22	9-TW	Permeable Asphalt (High Rise Residential 0.15 acre)	1	0.5	300	0.8	\$68,968	\$150	\$8,164	\$4,898
23	9-1W	Permeable Asphalt (High Rise Residential 0.1 acre)	1	0.3	200	0.6	\$47,188	\$100	\$8,366	\$5,577
24	ML-5	Grass Swale (East Moore Lake Park)	1	0.1	68	0.1	\$7,464	\$584	\$12,247	\$8,328
25	ML-3	New Wet Pond (Rice Creek Rd.)	1	0.6	251	0.0	\$240,448	\$450	\$33,725	\$14,108

Pollution reduction benefits and costs can not be summed with other projects in the same catchment because they are alternative options for treating the same source area.

Directly Connected Catchment
North Network

Catchment 8: Summary of preferred stormwater retrofit opportunities ranked by cost-effectiveness with respect to total phosphorus (TP) reduction. Retrofits within catchment 8 will benefit Innsbruck Nature Center. Volume and total suspended solids (TSS) reductions are also shown. For more information on each project refer to the catchment profile pages in this report

	atchment ID	Retrofit Type (refer to catchment profile pages for additional detail)	Projects Identified	TP Reduction (lb/yr)	TSS Reduction (lb/yr)	Volume Reduction (ac-ft/yr)	Total Project Cost	Estimated Annual Operations & Maintenance (2013 Dollars)	Estimated cost/ 1,000lb-TSS (30-year)	Estimated cost/ Ib-TP (30-year)
Nesidential Kain Gardens 1 - 5 0.4 - 1.5 0.4 - 1.5 0.4 - 1.5 0.4 - 1.5 0.7 - 54,709 - 54,700		Residential Rain Gardens	1 - 5	0.4 - 1.5	58 - 226	0.7 - 2.1	\$9,709 - \$34,38 <b>1</b>	\$75 - \$375	\$6,315 - \$6,873	\$960 - \$1,014

Disconnected Catchment

Catchments 1 through 6: Summary of preferred stormwater retrofit opportunities ranked by cost-effectiveness with respect to total phosphorus

Moore Lake Stormwater Retrofit Analysis

# References

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- Minnesota Stormwater Steering Committee. 2005. *Minnesota Stormwater Manual.* Minnesota Pollution Control Agency. St. Paul, MN.
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Appendix A: Methods Appendix A - Methods

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# **Methods**

# Selection of Subwatershed

Many factors are considered when choosing which subwatershed to analyze for stormwater retrofits. Water quality monitoring data, non-degradation report modeling, and TMDL studies are just a few of the resources available to help determine which water bodies are a priority. Stormwater retrofit analyses supported by a Local Government Unit with sufficient capacity (staff, funding, available GIS data, etc.) to greater facilitate the process also rank highly. For some communities a stormwater retrofit analysis complements their MS4 stormwater permit. The focus is always on a high priority waterbody.

For this analysis, areas draining to Moore Lake were chosen for study. Moore Lake is a high priority because it is classified as a Tier II water body by the Rice Creek Watershed District and is used regularly for recreation. Moore Lake was added to the EPA's 303(d) list of impaired waters for excess nutrients in 2002. Years of water quality monitoring identified increased levels total phosphorus and chlorophyll a that exceeded state standards.

Stormwater runoff from impervious surfaces like pavement and roofs can carry a variety of pollutants. While stormwater treatment to remove these pollutants is adequate in some areas, other areas were built before modern-day stormwater treatment technologies and requirements or have undersized treatment devices.



# **Stormwater Retrofit Analysis Methods**

The process used for this analysis is outlined in the following pages and was modified from the Center for Watershed Protection's *Urban Stormwater Retrofit Practices,* Manuals 2 and 3 (Schueler, 2005, 2007). Locally relevant design considerations were also incorporated into the process (*Minnesota Stormwater Manual*).

## **Step 1: Retrofit Scoping**

Retrofit scoping includes determining the objectives of the retrofits (volume reduction, target pollutant, etc.) and the level of treatment desired. It involves meeting with local stormwater managers, city staff and watershed management organization members to determine the issues in the subwatershed. This step also helps to define preferred retrofit treatment options and retrofit performance criteria. In order to create a manageable area to analyze in large subwatersheds, a focus area may be determined.

In this analysis, the focus area was all areas that drain to East and West Moore Lakes. Included are areas of residential, commercial, industrial, and institutional land uses. The subwatershed was divided into 10 catchments using a combination of existing subwatershed mapping data, stormwater infrastructure maps, and observed topography.

The targeted pollutant for this study was total phosphorus, though total suspended solids and volume were also modeled and reported. Total phosphorus (TP) was chosen as the primary target pollutant because long term water quality monitoring has identified elevated levels in East Moore Lake. Total suspended solids (TSS) was also reported because many other pollutants, such as heavy metals, are transported by these particles. Volume of stormwater was tracked throughout this study because it is necessary for pollutant loading calculations and potential retrofit project considerations.

## Step 2: Desktop Retrofit Analysis

The desktop analysis involves computer-based scanning of the subwatershed for potential retrofit catchments and/or specific sites. This step also identifies areas that don't need to be analyzed because of existing stormwater infrastructure or disconnection from the target water body. Accurate GIS data are extremely valuable in conducting the desktop retrofit analysis. Some of the most important GIS layers include: 2-foot or finer topography, hydrology, soils, watershed/subwatershed boundaries, parcel boundaries, high-resolution aerial photography and the stormwater drainage infrastructure (with invert elevations).

Feature	Potential Retrofit Project
Existing Ponds	Add storage and/or improve water quality by excavating pond bottom, modifying riser, raising embankment, and/or modifying flow routing
Open Space	New regional treatment (nond, bioretention).
Roadway Culverts	Add wetland or extended detention water quality treatment upstream.
Outfalls	Split flows or add storage below outfalls if open space is available.
Conveyance system	Add or improve performance of existing swales, ditches and non-perennial streams.
Large Impervious Areas (campuses, commercial, parking)	Stormwater treatment on site or in nearby open spaces.
Neighborhoods	Utilize right of way, roadside ditches, curb-cut rain gardens, or filter systems before water enters storm drain network.

## Desktop retrofit analysis features to look for and potential stormwater retrofit projects.

#### **Step 3: Retrofit Reconnaissance Investigation**

After identifying potential retrofit sites through this desktop search, a field investigation was conducted to evaluate each site and identify additional opportunities. During the investigation, the drainage area

and stormwater infrastructure mapping data were verified. Site constraints were assessed to determine the most feasible retrofit options as well as eliminate sites from consideration. The field investigation may have also revealed additional retrofit opportunities that could have gone unnoticed during the desktop search.

	Storm	water Treatment Options for Retrofitting
Area Treated	Best Management Practice	Potential Retrofit Project
cres	Extended Detention	12-24 hr detention of stormwater with portions drying out between events (preferred over wet ponds). May include multiple cell design, infiltration benches, sand/peat/iron filter outlets and modified choker outlet features.
-500 a	Wet Ponds	Permanent pool of standing water with new water displacing pooled water from previous event.
	Wetlands	Depression less than 1-meter deep and designed to emulate wetland ecological functions. Residence times of several days to weeks. Best constructed off-line with low-flow bypass.
	Bioretention	Use of native soil, soil microbe and plant processes to treat, evapotranspirate, and/or infiltrate stormwater runoff. Facilities can either be fully infiltrating, fully filtering or a combination thereof.
0.1-5 acres	Filtering	Filter runoff through engineered media and pass it through an under-drain. May consist of a combination of sand, soil, compost, peat, and iron.
	Infiltration	A trench or sump that is rock-filled with no outlet that receives runoff. Stormwater is passed through a conveyance and pretreatment system before entering infiltration area.
	Swales	A series of vegetated, open channel practices that can be designed to filter and/or infiltrate runoff.
	Other	On-site, source-disconnect practices such as rain-leader disconnect rain gardens, rain barrels, green roofs, cisterns, stormwater planters, dry wells, or permeable pavements.

#### General list of stormwater BMPs considered for each catchment/site.

#### **Step 4: Treatment Analysis/Cost Estimates**

Sites most likely to be conducive to addressing the cities' and watershed district's goals and appear to have simple-to-moderate design, installation, and maintenance were chosen for a cost/benefit analysis. Estimated costs included design, installation, and maintenance annualized across a 30-year period. Estimated benefits included are pounds of phosphorus and total suspended solids removed, though projects were ranked only by cost per pound of phosphorus removed annually.

## Treatment analysis

Each proposed project's pollutant removals were estimated using the stormwater model WinSLAMM. WinSLAMM uses an abundance of stormwater data from the upper Midwest and elsewhere to quantify runoff volumes and pollutant loads from urban areas. It is useful for determining the effectiveness of proposed stormwater control practices. It has detailed accounting of pollutant loading from various land uses, and allows the user to build a model "landscape" that reflects the actual landscape being considered. The user is allowed to place a variety of stormwater treatment practices that treat water

from various parts of this landscape. It uses rainfall and temperature data from a typical year, routing stormwater through the user's model for each storm.

The newest version of WinSLAMM (version 10), which allows routing of multiple catchments and stormwater treatment practices, was used for this analysis because of the unique connectivity amongst the catchments identified in the focus area under investigation. There are three areas where stormwater is routed through multiple catchments before being discharged to Moore Lake. This creates a network of stormwater treatment. Therefore, volume and pollutant loads to Moore Lake from any given catchment must take into consideration other treatment practices within the same network. The screen shot to the right displays the North Network of catchments used in this analysis to accurately model the effectiveness of the proposed BMP's while taking into account existing treatment from the pond on the north side of East Moore Lake (represented by "Wet Pond 2").

The initial step was to create a "base" model which estimated pollutant loading



from each catchment in its present-day state without taking into consideration any existing stormwater treatment. To accurately model the land uses in each catchment, we delineated each land use in each catchment using geographic information systems (specifically, ArcMap), and assigned each a WinSLAMM standard land use file. A site specific land use file was created by adjusting total acreage and accounting for local soil types (all soils were modeled as silt in this analysis). This process resulted in a model that included estimates of the acreage of each type of source area (roof, road, lawn, etc.) in each catchment. For certain source areas critical to our models we verified that model estimates were accurate by calculating actual acreages in ArcMap, and adjusting the model acreages if needed.

Once the "base" model was established, an "existing conditions" model was created by incorporating any existing stormwater treatment practices in the catchment. For example, street cleaning with mechanical or vacuum street sweepers, rain gardens, stormwater treatment ponds, and others were included in the "existing conditions" model if they were present in the catchment.

Finally, each proposed stormwater treatment practice was added to the "existing conditions" model and pollutant reductions were generated. Because neither a detailed design of each practice nor in-depth site investigation was completed, a generalized design for each practice was used. Whenever possible,
site-specific parameters were included. Design parameters were modified to obtain various levels of treatment. It is worth noting that we modeled each practice individually, and the benefits of projects may not be additive, especially if serving the same area. Reported treatment levels are dependent upon optimal site selection and sizing.

	General WinSLAMM Model Inputs
Parameter	File/Method
Land use acreage	ArcMap
Precipitation/Temperature	Minneapolis 1959 – the rainfall year that best approximates a
Data	typical year.
Winter season	Included in model. Winter dates are 11-4 to 3-13.
Pollutant probability	WI_GEO01.ppd
distribution	
Runoff coefficient file	WI_SL06 Dec06.rsv
Particulate solids	WI_AVG01.psc
concentration file	
Particle residue delivery	WI_DLV01.prr
file	
Street delivery files	WI files for each land use.

#### WinSLAMM stormwater computer model inputs

#### **Cost Estimates**

All estimates were developed using 2013 dollars. Cost estimates were annualized costs that incorporated design, installation, installation oversight, and maintenance over a 30-year period. In cases where promotion to landowners is important, such as rain gardens, those costs were included as well. In cases where multiple, similar projects are proposed in the same locality, promotion and administration costs were estimated using a non-linear relationship that accounted for savings with scale. Design assistance from an engineer is assumed for practices in-line with the stormwater conveyance system, involving complex stormwater treatment interactions, or posing a risk for upstream flooding. It should be understood that no site-specific construction investigations were done as part of this stormwater retrofit analysis, and therefore cost estimates account for only general site considerations.

The costs associated with several different pollution reduction levels were calculated. Generally, more or larger practices result in greater pollution removal. However the costs of obtaining the highest levels of treatment are often prohibitively expensive (see figure). By comparing costs of different treatment levels, the cities and watershed district can best choose the project sizing that meets their goals.



#### **Step 5: Evaluation and Ranking**

The cost per pound of phosphorus treated was calculated for each potential retrofit project. Only projects that seemed realistic and feasible were considered. The recommended level was the level of treatment that would yield the greatest benefit per dollar spent while being considered feasible and not falling below a minimal amount needed to justify crew mobilization and outreach efforts. Local officials may wish to revise the recommended level based on water quality goals, finances, or public opinion.

# Appendix B: How to Read Catchment Profiles

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## **Catchment Profiles and How to Read Them**

The analysis contains pages referred to as "Catchment Profiles." These profiles provide the most important details of this report, including:

- Summary of existing conditions, including existing stormwater infrastructure, and estimated pollutant export to Moore Lake
- Map of the catchment
- Recommended stormwater retrofits, pollutant reductions, and costs.

Following all of the catchment profiles (also in the executive summary) is a summary table that ranks all projects in all catchments by cost effectiveness.

To save space and avoid being repetitive, explanations of the catchment profiles are provided below. We strongly recommend reviewing this section before moving forward in the report.

The analyses of each catchment are broken into "base, existing, and proposed" conditions. They are defined as follows:

Base conditions -	Volume and pollutant loadings from the catchment landscape
	without any stormwater practices.
Existing conditions -	Volume and pollutant loadings after already-existing stormwater
	practices are taken into account.
Proposed conditions -	Volume and pollutant loadings after proposed stormwater retrofits

Analyses were performed at one of two geographic scales, "catchment or network." They are defined as follows:

Catchment level analyses -	Volume and pollutant loads exiting the catchment at the catchment
	boundary. There may be other stormwater practices existing or
	proposed farther downstream, but this analysis ignores them.
Network level analyses -	Volume and pollutant loads that reach Moore Lake through a
	stormwater network. One stormwater network consisting of three
	catchments (North Network) was identified in the Moore Lake
	subwatershed. Network loading estimates will be much larger than
	loading estimates from any one catchment because it is the sum of
	multiple catchments that discharge at the same point into the lake,
	and might receive treatment from the same practice. This analysis
	takes into account stormwater treatment ponds that are in-line with
	the conveyance system and upstream of Moore Lake. Catchments
	within a stormwater network will only have network level
	reductions reported in the catchment profile, since those reductions
	most accurately reflect the true cost-effectiveness of each project.

The pollutant load reduction for a single proposed stormwater retrofit will often be greater at the catchment level than at the network level. This is the result of existing treatment practices (such as a pond) located downstream that may have already been treating some of the pollutants being removed by a proposed project. For example, a proposed project may capture 10 pounds of phosphorus at the

catchment level, but that doesn't necessarily mean 10 fewer pounds of phosphorus will reach the creek because some of that phosphorus might have been removed by a network pond downstream. Benefits of a proposed project within a network must be judged by their pollutant reductions and cost effectiveness at the network level.

The example catchment profile on the following pages explains important features of each profile.

## **EXAMPLE Catchment A**

Existing Catchment	Summary	
Acres	58.90	
Dominant Land Cover	Residential	
Parcels	237	
Volume (acre-feet/yr)	18.37 、	
TP (lb/yr)	25.00	
TSS (lb/yr)	6461.00	

#### DESCRIPTION

Example Catchment is primarily comprised of mediumdensity, single-family residential development...

#### **EXISTING STORMWATER TREATMENT**

Existing stormwater treatment practices within Example Catchment consist of street cleaning with a mechanical sweeper in the spring and fall and a network of stormwater treatment ponds...

Catchment ID banner.

Volume and pollutants generated from this catchment under existing conditions, and excludes existing network-wide treatment practices

Catchment locator map.





Catchment Specific Existing Conditions

	Existing Conditions	Bas Load	e ing	Trea	itment	Т	Net reatment %	Existing Loading
	TP (lb/yr)	25.2		0.2			1%	25.0
nt	TSS (lb/yr)	7,18	36	725.0			10%	6,461
tme	Volume (acre-feet/yr)	18.	4	(	0.0		0%	18.4
rea	Number of BMP's					I		
	BMP Size/Description		Str	eet cle	et cleaning, st		rmwater po	ond



	Existing Conditions	Base Loading	Treatment	Net Treatment %	Existing Loading
	TP (lb/yr)	623.7	313.0	50%	310.7
nt	TSS (lb/yr)	216,101	124,172.0	57%	91,929
tm€	Volume (acre-feet/yr)	494.5	0.0	0%	494.5
rea	Number of BMP's	AI	BMPs in cate	chment netwo	ork
1	BMP Size/Description	tion Street cleaning and extended wet dete ponds just before outfall into target wat			

Same definitions as above, except here the numbers refer to pollutants and volumes discharged from the network collectively. The existing practices might include stormwater ponds that treat water from multiple catchments. These numbers reflect the cumulative impact of multiple catchments at the point they discharge to Coon Creek.

<text>

Proposed stormwater retrofits. The project ID number corresponds to this project's catchment and project type.

#### **RETROFIT RECOMMENDATIONS**

on Enhanced Sand Filte

New Piping to Detention Pond

BR = Bioretention (Rain Garden)

Wet Detention Pond

#### Project ID LCC-1 Residential RG's – Curb-Cut Rain Garden Network

1 000

Drainage Area – 33.7 acres Location – 5 locations throughout residential area Property Ownership – Private

Catch Basin

Outfall

Storm Sewer Lin

**Description** – The residential land cover within this catchment is best suited to residential, curb-cut rain gardens (see Appendix B for design options). Seven optimal rain garden locations were identified (see map below). Generally, ideal curb-cut rain garden locations are immediately up-gradient of a catch basin serving a large drainage area. Considering typical land owner participation rates we analyzed a scenario where 5 rain gardens were installed in catchment GL-3. Volume and pollutant reductions resulting from the rain garden installations are highlighted in the tables below.

\_\_\_Feet 2.000

#### EXAMPLE Conceptual and example images -



Before rain



During rain

Moore Lake Stormwater Retrofit Analysis

Ε	Appendix B – How to Read XAMPLE Catchment Specifi	Catchment Pr	ofiles t <b>Analysis</b>		HOW TO I	READ THE	CATCHME	WT DDAm.
	Vc th	lume or pollut s project will a	ant remova achieve.					NUF/LES
	Three "levels" of this proje compared: 6, 9, or 12 rain for example.	ect are gardens,				Cumulative removal ac this project already-exi practices.	e pollutant hieved by t and sting	
				Proje	ect ID	1		
	Cost/Benefit Analysis	6 Rain	Gardens	9 Rain (	Gardens	12 Rain Gardens		
		New trtmt	Net trtmt %	New trtmt	Net trtmt %	New trtmt	Net trtmt %	
	TP (lb/yr)	5.4	39%	6.8	43%	7.7	46%	
	TSS (lb/yr)	1,684	41%	2,127	45%	2,408	48%	
ut	Volume (acre-feet/yr)	4.2	33%	5.4	38%	6.1	41%	
mel	Number of BMP's		6	9	9	1	2	
Treat	BMP Size/Description	1,500	sq ft	2,250	sq ft	3,000	sq ft	
	ВМР Туре	Con Biore	Complex Bioretention		Complex Bioretention		nplex tention	
	Materials/Labor/Design	\$27	,210	\$40	,710	, \$54	,210	
	Promotion & Admin Costs	\$2,	\$2,450		\$2,870		290	
ost	Total Project Cost	\$29	,660	\$43	,580	\$57,500		
0	Annual O&M	\$4	50	\$6	575	\$9	900	
	Term Cost/lb-TP	\$8	355	\$1,	000 /	\$1,	170	
	Term Cost/1,000lb-TSS	\$2	266	\$3	313/	,\$3	864	

Project installation cost estimation.

Cost effectiveness at suspended solids removal. The project cost is divided by suspended solids removal in pounds (30 yrs). Includes operations and maintenance over the project life (30 years unless otherwise noted).

Cost effectiveness at phosphorus removal. The project cost is divided by phosphorus removal in pounds (30 yrs). Includes operations and maintenance over the project life (30 years unless otherwise noted).

#### Compare cost effectiveness

of various project "levels" in these rows for TP (2<sup>nd</sup> row from bottom) or TSS (bottom row) removal. Compare cost effectiveness numbers between projects to determine the best value.

E	XAMPLE Network-Wide Cost/	Benefit And	nlysis	App,	TO REAL	o Read Cato	hment Profiles			
			Project ID							
	Cost/Benefit Analysis	6 Rain (	Gardens	9 Rain (	Gardens	12 Rain	Gardens			
		New trtmt	Net trtmt %	New trtmt	Net trtmt %	New trtmt	Net trtmt %			
	TP (lb/yr)	5.4	39%	6.8	43%	7.7	46%			
	TSS (lb/yr)	1,684	41%	2,127	45%	2,408	48%			
t	Volume (acre-feet/yr)	4.2	33%	5.4	38%	6.1	41%			
men	Number of BMP's	6		9		1	2			
Treat	BMP Size/Description	1,500 sq ft		2,250 sq ft		3,000	sq ft			
T	ВМР Туре	Complex Bioretention		Complex Bioretention		Corr Bioret	plex ention			
	Materials/Labor/Design	\$27	,210	\$40,710		\$54	,210			
	Promotion & Admin Costs	\$2,450		\$2,870		\$3,	290			
ost	Total Project Cost	\$29	,660	\$43,580		\$57	,500			
Ö	Annual O&M	\$4	50	\$6	575	\$9	00			
	Term Cost/1,000lb-TSS/yr	\$8	55	\$1,	000	\$1,	170			
	Term Cost/lb-TP/yr	<b>\$</b> 2	66	\$3	63	\$4	14			

This table is the same as the previous catchment-level table, except it examines the costs and benefits of proposed stormwater retrofits at the network level. This table should be used to compare projects in catchments located in the North Network because it represents volume and pollutant removals at the point where the water enters Moore Lake.

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# Appendix C: Rain Garden Design Concepts

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## ANOKA COUNTY CURB-CUT RAINGARDENS



Drawing rainwater from the street gutter reduces runoff and pollutants to local water bodies



Prepared by the Anoka Conservation District in association with the Metropolitan Conservation Districts

## URBAN RAINWATER: SLOW IT DOWN AND SOAK IT UP

Under natural conditions the majority of rainwater falling on Anoka County would infiltrate the soil surface to be absorbed by plants or percolate more deeply into the soil to feed groundwater recharge and provide steady base-flow to streams and rivers. As land development has expanded more and more land is covered with impervious surfaces such as roads, parking lots and buildings. This conversion from native vegetation to impervious structure has greatly altered the hydrologic cycle and surface water ecology by greatly increasing runoff rates and effectively washing nutrient laden sediments and other pollutants into local surface waters. Treating and infiltrating urban rainwater as close to the point where it falls as possible is recognized as a vital and effective method for augmenting groundwater resources and reducing surface water quality impacts.

In dense residential **sub-watersheds** there is limited suitable public land on which to treat and infiltrate rainwater. In these situations utilizing private land and easements along roadways for treatment becomes an important tool for improving water quality. The curb and gutter system that channels rainwater quickly from your neighborhood can be disconnected with a *curb-cut* that directs rainwater from the street into a depressed *raingarden*. This allows rainwater falling within the catchment area of the raingarden to return to the natural hydrologic cycle of *infiltration* and *evapotranspiration*, effectively reducing downstream flooding, erosion and *non-point source pollution*. An individual curb-cut raingarden may only mitigate for a small portion of urban runoff, however the treating the rainwater runoff close to its source is an essential strategy in hydrologic restoration and cumulatively curb-cut gardens can actualize significant benefits within an urbanized *sub-watershed*.

The Anoka Conservation District has designed a set of curb-cut raingardens that can be applied to the physical conditions of your property and to your preference of garden shapes and plant selections. Each garden is designed to provide a water storage capacity of 100 cubic feet. Anoka Conservation



District has also designed a modular pretreatment box to be placed at the raingarden inlet to capture sediment and debris prior to water entering the garden. This pretreatment box is a vital component to the longevity and functionality of your raingarden. Please utilize the key on page 4 to determine the basic design needs of your property and continue to the designated page to select your choice of plant palettes. Plant images are shown of pages 20 and 21.



*curb-cut*: A section of curb and gutter that has been reconstructed to convey stormwater into a filter strip, rain garden, or other stormwater management strategy.

**evapotranspiration**: The transfer of liquid water from the earth's surface to atmospheric water vapor as result of transpiration by plants and evaporation by solar energy and diffusion. Evapotranspiration can constitute a significant water "loss" from a watershed.

*infiltration*: Water moving through a permeable soil surface by the force of gravity and soil capillary action. The rate of infiltration is highly dependent on soil type. Infiltration rates within the Anoka Sand Plain are generally very high.

**non-point source pollution**: Rainwater runoff that has accumulated pollutant loads (nutrients, sediments, petrochemicals etc.) over a large dispersed area. As opposed to point source pollution that has a defined single source.

**raingarden**: A landscaped garden in a shallow depression that receives rainwater runoff from nearby impervious surfaces such as roofs, parking lots or streets. The purpose of a raingarden is to reduce peak runoff flows, increase groundwater recharge and improve water quality in our lakes, streams and wetlands. Peak flow reduction is achieved by temporarily staging runoff within the raingarden basin until it infiltrates into the soil surface or evaporates (typically within 24 hours). This process also increases the quantity and movement of soil water that may feed groundwater recharge. Infiltrated water quality is improved by reducing sediment, nutrient and other chemical pollutant loads through chemical and biological processes in the soil. Downstream water quality is improved in kind by offsetting erosive peak flows and by capturing and treating pollutants higher in the watershed.

*sub-watersheds*: A discreet portion of a larger watershed, typically less than 2500 acres. Sub-watersheds can be more effectively analyzed and managed for water quality with site scale treatments.



## ANATOMY OF A CURB-CUT RAINGARDEN



## Raingarden Dimensions without a Retaining Wall



## Raingarden Dimensions with a Retaining Wall



## I. Rectangle Garden - Sunny Site - No Retaining Wall





#### II. Arc Garden - Sunny Site - No Retaining Wall





## III. Curvilinear Garden - Sunny Site - No Retaining Wall





## IV. Rectangle Garden - Shady Site - No Retaining Wall





## V. Arc Garden - Shady Site - No Retaining Wall





## VI. Curvilinear Garden - Shady Site - No Retaining Wall





## VII. Rectangle Ga rden - Sunny Site - Retaining Wall





## VIII. Arc Ga rden - Sunny Site - Retaining Wall





## IX. Curvilinear Ga rden - Sunny Site - Retaining Wall





## X. Rectangle Garden - Shady Site - Retaining Wall



## XI. Arc Garden - Shady Site - Retaining Wall





## XII. Curvilinear Garden - Shady Site - Retaining Wall







Lobelia cardinalis

SENSITIVE FERN Onoclea sensibilis GOLDSTRUM BLACK-EYED SUSAN Rudbeckia fulgida CULVERS ROOT Veronicastrum virginicum



Plant pallette





Plant pallette



KARL FORESTER GRASS Calamagrostis acutifolia



Carex pennsylvanica



FOX SEDGE Carex vulpinoidea



JUNE GRASS Koeleria macrantha

LITTLE BLUESTEM Schizachyrium scoparium



Sporobolis heterolepsis

Intentionally Blank
# Appendix D: Retrofit Concepts

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Prepared by the Anoka Conservation District in association with the Metropolitan Conservation Districts

## **Perimeter Sand Filter**

**Perimeter sand filters** (Delaware filters) consist of two parallel trench-like chambers that are typically installed along the perimeter of a parking lot. Parking lot runoff enters the first chamber, which has a shallow permanent pool of water. The first trench captures heavy solids before the runoff spills into the second trench, which consists of a sand layer (typically 18" deep). Water infiltrates through the sand and is collected by an under-drain and delivered, ideally, to another stormwater BMP or existing stormsewer network. If both chambers fill up to capacity, excess parking lot runoff is routed to a bypass drop inlet. The sand may have iron filings added to improve dissolved phosphorus removal.

### **BENEFITS:**

- Great for adjacent to large impervious areas like parking lots
- Remove up to 90 percent of total suspended solids, 55 percent of total phosphorous, and 35 percent of total nitrogen
- Can effectively treat hot-spot runoff
- Consume small amounts of land

### COST:

• Approximately \$21.50 per cu ft of storage

# Sand filter inspection, Iowa Stormwater Partnership

### CONCERNS:

- High maintenance burden (regular inspections for clogging, sand replacement, and removal of captured sediment)
- Not recommended for areas with high sediment content in stormwater or areas receiving significant clay/silt runoff
- · Relatively costly

### RECOMMENDED DRAINAGE AREA:

- · Highly impervious sites up to 2 acres
- Approximately 100 linear feet treats 1 acre of impervious area



# **Tree Pit Filter**

**Stormwater tree pits** consist of an underground structure and above ground plantings which collect and treat stormwater using bioretention. Although their structures differ, stormwater tree pits closely resemble traditional street trees and are perfect for urban streets where space is limited.

### BENEFITS:

- Reduces runoff volume, flow rate and temperature
- Increases groundwater infiltration and recharge
- Improves aesthetic appeal of streets and neighborhoods
- Provides shade to nearby buildings to reduce energy costs
- Requires limited space
- Simple to install
- Available in multiple sizes
- Eliminates watering and fertilizing needed by traditional street trees

### CONCERNS:

- Tree species will be limited to those that have salt tolerance and limited root aggression
- Regular inspections to prevent clogging & maintain function



RECOMMENDED DRAINAGE AREA:

• Optimum ratio at highy impervious sites is one 6' x 6' tree pit per .25 acres

### COST:

Approximately \$98.75 per cu ft of storage



**Porous pavements** come in a wide array of materials - *concrete, asphalt, pavers, and grid* - with void spaces that allow water to percolate through the surface and reach a subsurface layer of coarse aggregate allowing stormwater to quickly drain into the ground. Porous pavements are ideally situated in areas where soil type, seasonal water table and frost line levels allow for groundwater recharge. Porous pavements are typically used in low traffic areas and are well suited for use in parking lots, overflow areas, low traffic roads, residential driveways and pedestrian walkways. They can also be installed surrounding other stormwater management systems to provide overflow collection and infiltration.

### BENEFITS:

- Reduces runoff volume, flow rate and temperature
- · Increases groundwater infiltration and recharge
- Reduces the need for traditional stormwater infrastructure
- Can improve aesthetic appeal of paved areas (pavers)
- Flexible for use in areas of various shapes and sizes
- Remove up to 80 percent of total phosphorous and total nitrogen
- Reduced Ice buildup on street

### CONCERNS:

- Typically not suited for slopes greater than 5%
- Cost
- At minimum 2 vacuum sweepings per year
- Periodic replacement of fill material in joint spacing (pavers)
- Not suitable for areas generating a lot of sediment

### RECOMMENDED DRAINAGE AREA:

• Typically 3:1 (drainage area to porous pavement area) or less

### COST:

• Approximately \$14 - \$35 per cu ft storage depending on underlayment



# **Porous Pavement**







**Flow Splitters** 

**Flow splitters** are stormsewer structures used to divert initial flows from stormsewer network out into a stormwater BMP such as constructed wetlands, detention ponds, infiltration basins, swales and various other filtration practices. During intense rain events excess stormwater travels over a weir, located in the flow splitter, and continues down pipe. Flow splitters are often designed to divert at least the 'first flush' into a BMP.

### BENEFITS:

- Provides the ability to capture and treat otherwise untreated stormwater
- Allows high flows to bypass the connected stormwater BMPs thus reducing opportunities for erosion and re-suspension of sediment captured in the BMP systems
- Only periodic inspections are needed, with annual debris / sediment cleanout being sufficient

### CONCERNS:

• Alone this practice does not reduce pollutants. It is a tool to divert appropriate flows into a water quality practice



RECOMMENDED DRAINAGE AREA:

• Varies, pipe sizing can be scaled according to drainage area and capacity of Stormwater BMP that flow is diverted to

### COST:

• Varies, the smallest typical structure to fit a weir is 48" diameter.

- Individual component costs of a 48" diameter structure\*:
  - 1. Base slab ~ \$250,
  - 2. Weir ~ \$200 per vertical foot,
  - 3. Riser (side walls) ~ \$130 per vertical foot,
  - 4. Cover slab (with opening) ~ \$300,
  - 5. Metal casting (top grate, option) ~ \$400
  - 6. Diverted flow pipe ~ \$2 \$10 per linear foot

(depends on material and diameter)

\*Based on local sourcing, 2010



# Hydrodynamic **Separators**

Hydrodynamic Separator devices are structural BMPs vary in size and function, but all use some form of filtration, settling, or hydrodynamic separation to remove particulate pollutants from overland or piped flow. They often replace traditional catch basins and look much the same from the surface. Below the surface is a series of baffles, chambers, and devices designed to capture pollutants. They generally remove coarse sediment, oil and grease, litter, and debris and are often employed in areas with high concentrations of pollutants in runoff (ultra urban and retrofit situations). They may serve as pre-treatment of stormwater runoff before it reaches other BMPs, such as infiltration systems. Manufacturers of the devices provide the internal design specifications and installation instructions.

### **BENEFITS:**

- · Can be used in a variety of applications including retrofitting existing stormwater systems
- Subsurface device, consumes little to no land
- · Removal of sediment, oils and other floatables

### CONCERNS:

 A minimum annual vacuum removal of captured pollutants; however, required inspections every 6 months for the first year observing sedimentation and oil accumulation rates may determine more frequent visits are necessary

High initial installation costs

### **RECOMMENDED DRAINAGE AREA:**

 With a suite of scalable devices, drainage areas can range from a single parking lot up to 7 acres of predominently impervious surfaces (based on a standard 80% removal rate of total suspended solids on Stormceptor products\*\*)

### COST:

• Varies widely, from \$2,300 to \$40,000 depending on site characteristics including the amount of runoff (in cfs) required to be treated, the amount of land available, and any other treatment technologies that are presently being Often costs break down to used. approximately \$9,000 per acre runoff treated\*



\*EPA Technology Fact Sheet \*This mention does not constitute an endorsement of product



